EVT / Kosciuszko Thredbo Pty Ltd

Proposed Alpine Coaster Thredbo NSW

Geotechnical Investigation

Our ref: 5920-G1 Rev 3
11 August 2022

Your trusted engineering professionals

Document Authorisation

Prepared for EVT / Kosciuszko Thredbo Pty Ltd

Our ref: 5920-G1 Rev 3
11 August 2022

For and on behalf of
AssetGeoEnviro

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Document Control

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Contents

1. Introduction

1.1 General

This report presents the results of a geotechnical investigation for the above project. The investigation was commissioned on 27 January 2021 by Mr Peter Fleming of EVT / Kosciuszko Thredbo Pty Ltd, purchase order KTM038014. The work was carried out in accordance with the proposal by AssetGeoEnviro (Asset) dated 18 January 2021, reference 5920-P1 Rev 1.

Documents supplied to us for this investigation comprised:

- Photograph showing typical coaster (unreferenced, undated).
- Structural Analysis and Design (prepared by: Wiegand; reference: 523a/11; dated: 3 July 2012).
- Foundation Drawings (prepared by: Wiegand; reference: FD-KR0-008_EU and 011_EU; dated: 1 July 2015; FD-MST-Typ1_EU, Typ2_EU, Typ3_EU; dated 25 February 2015).
- Trench Section & Steel Pipe Sizing Plans (prepared by EVT/ Kosciuszko Thredbo Pty Ltd, undated).
- Architectural Plans (prepared by: DJRD Architects; project: 20413; drawing numbers: A1.001(B), A1.218(B), A1.219-A(B), A1.219-B(B), A1.219-C(B), A1.219-D(B), A1.220(B), A1.221(B), A1.222(B), A1.223(B), A1.224(A), A1.225(A), A1.226(B), A1.227(B), A1.228(B), A1.235(B); issued: 14 April 2022).

We understand that the project involves construction of a Coaster Ride above the existing ground surface supported on a series of steel columns and frames as illustrated in Plate 1. The proposed Alpine Coaster is over 1400m in length, comprising an uphill track about 380m length and downhill track of over 1000m length. The elevation difference over the route is about 130m. The bottom station comprises a base building, tunnel, and associated works.

We also understand that the development will require the installation of new and relocation of existing snowmaking infrastructure nearby the Merritts Gondola and Snowgums Chairlift base stations (refer attached Snowmaking Layout plan in Figures). All The works will comprise:

- Removal of existing snowmaking lateral;
- Trenching for laying of pipes, pits and electrical and communication cable;
- Installation of new snowmaking pipe to connect to existing and new valve pits;
- Installation of six (6) new valve pits and guns; and
- Installation of one (1) new manual hydrant.

The trenches will be approximately 0.8m wide x 0.6m to 0.8m deep. The distribution footprint of the pits supporting the snowmaking guns will be approximately 3 m x 3 m wide x 1.6 m deep.

Footings for circles, bridges, and mono supports are proposed to comprise reinforced concrete pad footings designed for a maximum allowable bearing pressure of 150 kPa. Footings for other elements comprise steel plates with typical dimension of 250mm x 250mm with ground stakes at each corner, imposing a bearing pressure of approximately 176 kPa. Typical supports are shown on Plate 2.

A tunnel section is also proposed on the start of the uphill part of the track, constructed as six precast concrete culvert sections laid in an open trench and filled over. It is understood that excavation of up to about 3m to 4m is proposed for the tunnel section.

Plate 1: Typical Coaster

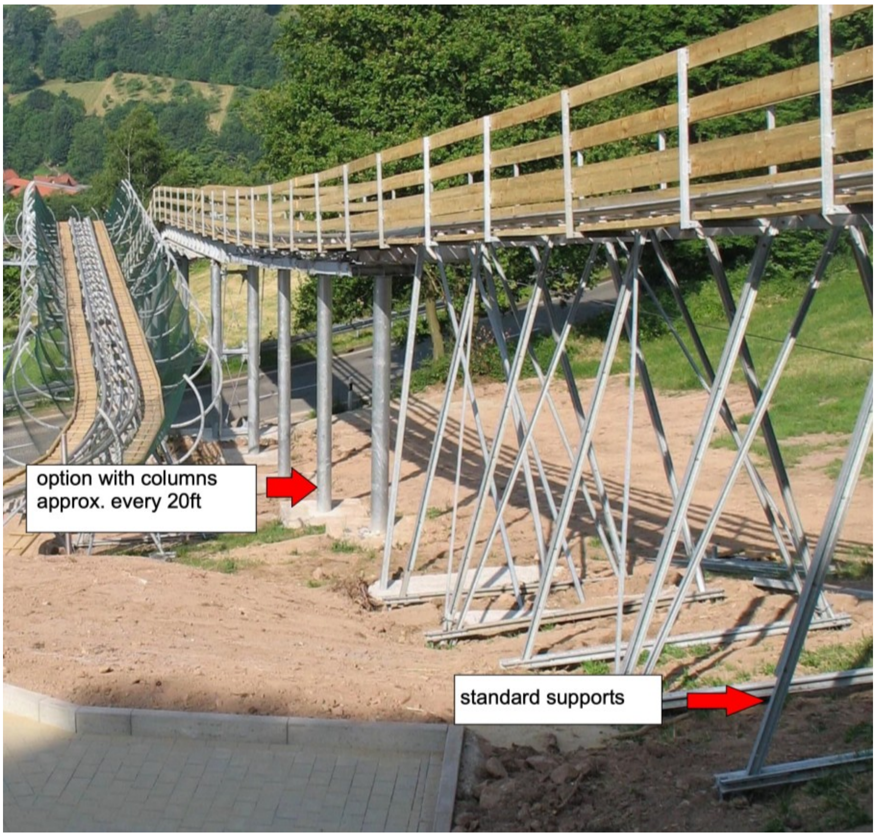

Plate 2: Typical Supports

1.2 Scope of Work

The main objectives of the investigation were to assess the surface and subsurface conditions and to provide comments and recommendations relating to:

- Landslide risk assessment as per AGS 2007[1].
- Key geotechnical constraints to the development.
- Excavation conditions, methodology and monitoring.
- Subgrade preparation and earthworks.
- Suitable foundation options.
- Allowable bearing pressure (and shaft adhesion for shallow pile-type footings).
- Anticipated settlements.
- Maximum allowable permanent and temporary batter slopes.

The following scope of work was carried out to achieve the project objectives:

- A review of existing regional maps and reports relevant to the site held within our files.
- Clearance of underground services at proposed test locations.
- Visual observations of surface features.
- Subsurface investigation at 29 locations to sample and assess the nature and consistency of subsurface soils and bedrock at accessible areas of the site.
- Engineering assessment and reporting.

This report must be read in conjunction with the attached "Important Information about your Geotechnical Report" in Appendix A. Attention is drawn to the limitations inherent in site investigations and the importance of verifying the subsurface conditions inferred herein. Slope instability considerations presented in this report must be read in conjunction with the attached GeoGuides for Slope Management and Maintenance.

[1] Landslide Risk Management, Australian Geomechanics, Vol 42, No. 1, March 2007.

2. Compliance with Geotechnical Policy

The following table indicates how this report addresses the elements in Section 4.1 of the Department of Planning Industry & Environment's Geotechnical Policy (Policy):

Paragraph	Element	Addressed in Report
4.1 (a)	An assessment of the risk posed by all reasonably identifiable geotechnical hazards which have the potential to either individually or cumulatively impact upon people or property upon the site or related land to the proposed development in accordance with the guidelines set out in 'Landslide Risk Management Concepts and Guidelines' first published in the Australian Geomechanics Journal, Vol. 35 No.1, March 2000 (guidelines).	Risk assessment has been carried out by a highly experienced geotechnical practitioner and outlined in Tables A and B attached to this report.
4.1 (b)	Plans and sections of the site and related land from survey and field measurements with contours and key features identified, including the locations of the proposed development, buildings/structures on both the subject site and adjoining site, stormwater drainage, sub-surface drainage, water supply and sewerage pipelines, trees, and other identifiable geotechnical hazards.	Figure 1 – Site Locality Plan Figure 2 – Test Locations
4.1 (c)	Details of all site inspections and site investigations and any other information used in preparation of the geotechnical report. A site inspection is required in all cases. Site investigation may require subsurface investigation; appropriate investigation may involve boreholes and/or test pit excavations or other methods necessary to adequately assess the geotechnical/geological model for the site. At Thredbo, reference may be made to the suite of existing geotechnical data and regional studies held by Kosciuszko Thredbo Pty Ltd, as part of the information to be used in assessing the site. Where similar information data exists for the other Kosciuszko Ski Resorts then this information may be similarly used in assessing the site.	Test pit, borehole, and DCP logs provided in Appendix B, including Soil & Rock Explanation Sheets.
4.1 (d)	Photographs and/or drawings of the site and related land adequately illustrating all geotechnical features referred to in the geotechnical report, as well as the locations of the proposed development.	Photos included in Appendix C, figures listed in response to 4.1 (b).
4.1 (e)	Presentation of a geological model of the site and related land showing the proposed development, including an analysis of sub-surface conditions, considering thickness of the fill, alluvium, topsoil, colluvium, and residual soil layers, depth to underlying bedrock, and the location and depth of groundwater.	Refer Section 6.
4.1 (f)	A conclusion as to whether the site is suitable for the development proposed to be carried out either conditionally or unconditionally.	Refer Section 8.

Paragraph	Element	Addressed in Report
4.1 (f) (i), (ii), (iii)	Conditions to establish the design parameters, conditions applying to the detail design to be undertaken for the construction certificate, conditions applying to the construction phase, and conditions regarding ongoing management of the site/structure.	Refer Section 10. A qualified and experienced Geotechnical Engineer is to be engaged to provide further input and review during the design development; including site visits during construction to verify the site conditions and provide advice where conditions vary from those assumed in this report. We herewith confirm that inspection may also be carried out by suitably experienced site personnel under direction of an experienced geotechnical engineer. No specific conditions are identified for ongoing management of the site / structure.

3. Regional Topography

The regional topography comprises moderately to steeply sloping terrain upslope of the north-easterly flowing Thredbo River. Ground slopes over the alignment generally range from 8º to 17º and some locally steeper sections up to about 25º to 30º.

The site lies within an area designated as "G" as defined in the maps accompanying DIPNR's "Geotechnical Policy – Kosciuszko Alpine Resorts", November 2003, and therefore a geotechnical report is required to accompany the development application as per the requirements of the Geotechnical Policy.

4. Site Description

The site is located within Thredbo, northwest of the Alpine Way and the Thredbo River and north of the Valley Terminal as shown in Figure 1.

The ground surface slopes up to the northwest at an average of about 17º, locally flatter over the downhill part on the northern flanks of the Thredbo River and with locally steeper slopes of up to about 25º to 30º.

The exit / entry area is located near a tennis court to the east of the Valley Terminal, as shown in Photo 1. The route then turns to the northwest (beginning of uphill section) and approaches the tunnel section beneath the existing Gondola run (see Photo 4). After exiting the tunnel, the coaster incline proceeds northwest up to the top of the run approximately 330m from the start. The uphill section is located parallel to an existing ski lift.

The downhill section is situated to the north of the uphill section and winds down the hillside to end at a double loop near the tennis court.

Some of the route is across cleared areas that are covered with low grasses. The downhill section travels through a mixture of thickly vegetated uncleared bush and cleared areas.

No rock outcrop was observed across the route. Occasional partly buried granite boulders were observed at various locations.

Surface drainage is by overland flow occasionally concentrated in seasonally wet areas.

The landform in the lower part of the site including the double-loop end of the downhill section, entry/exit station, and start of the uphill section, corresponds to the northern flank of the Thredbo River, and appears to have been filled to create level areas, with the depth of fill expected to increase with proximity to the river.

5. Fieldwork

The fieldwork was undertaken on 21 and 22 January 2021 under the full-time supervision of a Senior Principal Geotechnical Engineer from Asset and included invasive investigation at 29 locations, 25 by test pit and 4 by hand auger and Dynamic Cone Penetrometer (DCP) sounding.

The test locations are shown in the attached Figure 2 were set out by our Senior Principal Geotechnical Engineer relative to existing site features.

Buried metallic services and utilities within the site boundaries near critical test locations (i.e., close to know buried services) were cleared by a service locator from Kosciuszko Thredbo Pty Ltd and by referring to utility maps held by Kosciuszko Thredbo Pty Ltd.

The invasive investigation included excavation of test pits at 25 locations using a Kubota U17–3 excavator, terminated at depths ranging from 0.6m to 2.0m, and drilling 4 hand auger boreholes with DCP soundings to depths ranging from 0.7m to 1.7m.

Engineering logs are provided in Appendix B together with their explanatory notes.

6. Subsurface Conditions

6.1 Geology

The 1:250,000 Tallangatta Geological Map indicates the site is underlain by Silurian aged intrusive granite.

6.2 Subsurface Conditions

The testing indicated that the subsurface materials are quite variable, and include the following generalised geotechnical units as summarised below. For a detailed description of the subsurface conditions, refer the attached engineering logs and explanatory notes. For specific design input, reference should be made to the logs and/or the specific test results, in place of the following summary.

Summary Geotechnical Units

Fill: comprising mixture of GRAVELS and COBBLES up to 150 mm size in a Silty Clayey SAND/Sandy CLAY/SILT matrix, moist, medium dense to very dense/stiff to hard, appears to be moderately compacted to well compacted.

Alluvium: comprising Sandy Clayey SILT, trace organic matter, moist greater than plastic limit, stiff to very stiff.

Topsoil: comprising Silty SAND with organic matter / tree roots, fine grained, dry to moist, loose to medium dense.

Slopewash: comprising Sandy Clayey SILT and Silty CLAY, medium plasticity, dark brown, moist less than plastic limit, very stiff to hard.

Residual: comprising Silty SAND / Clayey SAND / Sandy CLAY / Silty CLAY, moist / greater than plastic limit to wet, medium dense to dense / stiff to hard.

CW Granite: comprising SAND and Clayey SAND, medium to coarse grained, moist to wet, medium dense to very dense.

Extremely to Highly Weathered Granite: comprising blocks of granite up to 150 mm size within a Silty SAND / Clayey SAND matrix.

The site can be delineated into three areas for geological profile description as follows: Upper Slope (TP01 to 14, 21 to 25); Lower Slope (TP15 to 18); and Tunnel Section (TP19 and 20).

Upper Slope (TP01 to 14, 21 to 25)

The geological profile for the Upper Slope generally comprises topsoil overlying slopewash overlying residual overlying weathered Granite. The depth to the top of the slopewash and residual soils varies from about 0.3 m to 0.8 m. The depth to the weathered Granite generally varies from about 0.25 m up to about 1.5 m.

Groundwater was generally not observed at the test locations within the Upper Slope except at TP 05 where slow groundwater inflow at 0.5 m depth was noted.

Lower Slope (TP15 to 18)

The geological profile for the Lower Slope generally comprises fill overlying topsoil at three locations overlying alluvial soils at two locations, overlying residual soils. The weathered Granite was generally not encountered at the test locations within the Lower Slope which extended to depths ranging from 1.5 m up to about 2.5 m. The depth to the top of the alluvial and residual soils varies from about 0.3 m to 1.2 m.

Groundwater was observed within the lower slope at TP18 where moderate groundwater inflow at 1.0m depth was noted.

Tunnel Section (TP19 & 20)

The geological profile for the Tunnel Section generally comprises fill and alluvial soils to about 0.9m depth in TP19 and fill to about 1.3m depth in TP20, overlying residual soils / completely weathered Granite to depths of at least 2.5m.

Groundwater was not observed in TP19 within the 2m depth of excavation, and rapid groundwater inflow was observed in TP20 at 1.3m depth.

The depths and unit thicknesses are based on the information from the test locations only and do not necessarily represent the maximum and minimum values across the site.

7. Discussions & Recommendations

7.1 Key Geotechnical Site Constraints

The development will generally require modest bearing capacity for the footings for the coaster which is generally available at shallow depth. Geotechnical constraints include variable depths to suitable founding stratum for these footings and hazards relating to landslide risk.

Geotechnical constraints for the tunnel section include variable excavation conditions including potentially harder rock below the depths of investigation, and groundwater control for excavations.

Recommendations for design and construction of the development are provided in the following sections.

7.2 Landslide Risk Assessment

A landslide risk assessment has been carried out for this site using the methods of AGS 2007[2].

The basis of the preliminary assessment undertaken for this site and important factors relating to slope conditions and the impacts of the development that commonly influence landslide risks are discussed in the attached "Important Information about your Landslide Risk Assessment", and the attached GeoGuides.

The preliminary assessment has been carried out by:

- Consideration of the likely slope failure mechanisms and the likely initiating circumstances that could affect the elements at the site. The type and mode of landslide failure has also been classified.
- **Risk to Property.** For each case, the likely consequences with respect to future development have been considered. The current assessed probability of occurrence of each event has been estimated on a qualitative basis. The consequences and probability of occurrence have been combined for each case to provide the risk assessment.
- **Risk to Life.** For each case, the risk for the person most at risk is assessed based on multiplying the indicative annual probability of the occurrence of the hazard, the probability of spatial impact, the

[2] Landslide Risk Management, Australian Geomechanics, Vol 42, No. 1, March 2007.

temporal probability, the vulnerability, and the probability of not evacuating. The risk is then compared with acceptable and tolerable risk criteria.

The following general potential hazards/events are identified for this site and relate to slope instability:

A generic shallow slump affecting coaster footing

B generic translational slide affecting coaster footing

We note that the proposed excavation for the tunnel will involve a nominal 4m wide (base width) trench directly aligned upslope and with a gentle to moderate gradient (i.e., not a steep cutting) to transition from the tunnel section to the existing ground surface. This excavation is therefore assessed to present minimal potential landslide risk during the construction.

For the hazards / events identified, the elements that are at risk are the proposed coaster footings along the routed. Table A provides our preliminary risk assessment for the site with respect to risk to property, and Table B provides our preliminary risk assessment for the site with respect to risk to life.

Where development takes into consideration the possible failure mechanisms and adopts good engineering practice for hillside development, it is envisaged that the outcome of such a development would be a **Low** risk assessed with respect to property and the risk with respect to life would be **Acceptable.**

The development should be carried out in accordance with good engineering practice that is described in the attached GeoGuides, and in accordance with the general recommendations in the following sections.

7.3 Footings

The following recommendations are provided for footings. An experienced Geotechnical Engineer should review footing designs to check that the recommendations of the geotechnical report have been included.

7.3.1 Coaster Alignment

The coaster footings should be founded within stiff / medium dense or better natural soils, below any fill and topsoil, and below softer / looser soils. A maximum allowable bearing pressure of 180kPa may be adopted for design purposes where footings meet these criteria.

The depth of footing excavations will vary depending on local conditions at each footing. It is anticipated that many of the footings could be dug to suitable founding material by hand methods, but some mechanical digging should be anticipated.

7.3.2 Tunnel Section

The tunnel section will likely be down into weathered granite at its deepest point. Loadings from the tunnel section are anticipated to be relatively small but there could be significant differential settlement if variable foundation material is adopted. We recommend the entire tunnel section be founded on the weathered granite anticipated to be encountered within the deepest sections. This could be achieved by shallow footings where this material is exposed at bulk excavation level, or piles where it is not. For preliminary design, an allowable bearing pressure of 600kPa may be adopted for highly weathered granite or better, to be confirmed by further investigation or inspection during construction.

7.3.3 Exit / Entry Area

High level footings could be adopted for the exit / entry area, founded within very stiff / dense or better natural soils at nominally 0.5m depth below existing ground surface, and designed for a maximum allowable bearing pressure of 200kPa.

7.4 Earthworks

7.4.1 Excavation

The excavation for the footings and service trenches is anticipated to be predominantly with soils and completely weathered granite (remoulds to medium to coarse sands) of variable nature and composition as indicated by the testing. Deeper excavation for the tunnel section would likely involve excavation within less weathered granite that would require hammering or blasting. Some larger cobbles and boulders should be anticipated, which could be removed with suitably sized excavators.

7.4.2 Subgrade Preparation

The following general recommendations are provided for subgrade preparation:

- Excavate to design subgrade level and foundation material required for each footing. Remove unsuitable materials from the site (e.g., material containing deleterious matter). Stockpile topsoil and organic matter for re-use as landscaping material, or remove from site, or spread nearby as per civil plans. Residual soils and completely weathered granite, and suitable fill soils could be stockpiled for re-use as engineered fill or removed to spoil.

Any waste soils being removed from the site must be classified in accordance with current regulatory authority requirements to enable appropriate disposal to an appropriately licensed landfill facility. Asset can provide further advice on this matter if required.

7.4.3 Filling

Filling is not anticipated to be required except above the tunnel section. Filling for the tunnel section, and other filling if needed, should be placed in horizontal layers over prepared subgrade and compacted as per Table 1. Filling on existing slopes should not exceed 1m vertical height above existing surface level without further geotechnical advice.

Table 1 – Compaction Specifications

Parameter	Cohesive Fill	Non-Cohesive Fill
Fill layer thickness (loose measurement): • Within 1.5m of the rear of retaining walls • Elsewhere	 0.2m 0.3m	 0.2m 0.3m
Density: • Beneath Pavements • Beneath Structures • Upper 150mm of subgrade	 ≥ 95% Std ≥ 98% Std ≥ 100% Std	 ≥ 70% ID ≥ 80% ID ≥ 80% ID
Moisture content during compaction	± 2% of optimum	Moist but not wet

Filling within 1.5m of the rear of any retaining walls and tunnel section should be compacted using lightweight equipment (e.g., hand-operated plate compactor or ride-on compactor not more than 3 tonnes static weight) to limit compaction-induced lateral pressures. This must be checked with the tunnel designer to ensure the tunnel structure is not overloaded.

Any soils to be imported onto the site for backfilling and reinstatement of excavated areas should be free of contamination and deleterious material and should include appropriate validation documentation in accordance with current regulatory authority requirements which confirms its suitability for the proposed land use. Asset can provide further advice on this matter if required.

Filling to create new batter slopes should be over-placed (i.e., extend beyond the design line and level), compacted, then trimmed back to the design line and level, so that the trimmed surface contains compacted material that conforms to the above specification.

7.4.4 Batter Slopes

Given the shallow nature of excavation for the coaster footings, anticipated to be not more than about 1m deep), practically, vertical cuts may be carried out if sidewalls remain stable during construction.

Temporary battering will be required for the tunnel section. Batters should be no steeper than 1H:1V with intermediate benches of minimum 0.5m width at nominal 2m maximum lift heights. If competent granite is encountered that requires hammering or blasting, this may be cut at no steeper than 0.25H:1V. Inspection of the excavation must be carried out by a suitably experienced geotechnical engineer before personnel access is allowed in the trench. Alternatively, temporary shoring may be provided either by shoring system designed by a Structural Engineer or by a proprietary shoring system certified by a Structural Engineer for the anticipated site conditions.

7.5 Groundwater Control

Limited groundwater observations made for this investigation are described in Section 6.2. The observations indicate that groundwater is unlikely to be a constraint to the majority of the coaster route, except at the tunnel section where rapid inflow was observed. This will require diversion trenching and pumping to allow the tunnel section to be constructed.

There may also be localised springs elsewhere that affect footing construction (e.g., TP 05), which may require localised diversion drain.

Good practice should be followed if groundwater seepage is encountered during construction, such as installation of subsoil drainage. Further geotechnical advice must be sought if significant groundwater is encountered during construction that cannot be controlled using diversion drains.

8. Site Suitability

We herewith conclude that the site is geotechnically suitable for the development provided that the development is carried out in accordance with the recommendations and advice in this report including the following Development Approval Conditions.

9. Further Investigation

It is noted that details of the tunnel including depths of excavation were advised to Asset at the time of the fieldwork, which had not planned subsurface investigations to the depths involved. Further investigation is recommended to verify the subsurface conditions as part of detailed design, or as a minimum, before construction commences. This should involve drilling or boreholes to depths of at least 6m or refusal, whichever occurs first, or deep test pitting to at least the invert level of the tunnel section.

10. Recommended Development Approval Conditions

The following conditions should be included with the development approval:

(i) Conditions to be provided to establish the design parameters

The development shall be carried out in accordance with the requirements and recommendations of the Geotechnical Investigation by AssetGeoEnviro dated 11 August 2022 (Ref 5920-G1 Rev 3), and in accordance with further geotechnical investigation and advice to be provided during design development and before construction as required.

(ii) Conditions applying to the detailed design to be undertaken for the construction certificate

Structural design and civil details (drainage, earthworks) relating to the geotechnical aspects of the proposed development shall be checked and certified by a suitably qualified and experienced Geotechnical Engineer as being in accordance with the geotechnical recommendations.

(iii) Conditions applying to the construction

During construction, inspection shall be carried out by a suitably qualified and experienced geotechnical engineer or by suitably experienced site personnel under direction of an experienced geotechnical engineer, at the following stages, to ensure that the requirements of the geotechnical report are followed:

a) Footing excavations shall be inspected after preparation. Inspection shall be by an experienced Geotechnical Engineer, either in-person or by remote inspection (e.g., via video) with proof testing as directed by the Geotechnical Engineer.

b) Temporary batter slopes for the tunnel section shall be inspected after excavation.

(iv) Conditions regarding ongoing management of the site/structure

No specific conditions are identified for ongoing management of the site / structure.

11. Limitations

In addition to the limitations inherent in site investigations (refer to the attached Information Sheets), it must be pointed out that the recommendations in this report are based on assessed subsurface conditions from limited investigations.

It is recommended that a qualified and experienced Geotechnical Engineer be engaged to provide further input and review during the design development; including site visits during construction (either in-person or by remote with assistance of suitably experienced site personnel) to verify the site conditions and provide advice where conditions vary from those assumed in this report.

This report may have included geotechnical recommendations for design and construction of temporary works (e.g., temporary batter slopes or temporary shoring of excavations). Such temporary works are expected to perform adequately for a relatively short period only, which could range from a few days (for temporary batter slopes) up to six months (for temporary shoring). This period depends on a range of factors including but not limited to: site geology; groundwater conditions; weather conditions; design criteria; and level of care taken during construction. If there are factors which prevent temporary works from being completed and/or which require temporary works to function for periods longer than originally designed, further advice must be sought from the Geotechnical Engineer.

This report and details for the proposed development should be submitted to relevant regulatory authorities that have an interest in the property (e.g., Department of Planning) or are responsible for services that may be within or adjacent to the site for their review.

Asset accepts no liability where our recommendations are not followed or are only partially followed. The document "Important Information about your Geotechnical Report" in Appendix A provides additional information about the uses and limitations of this report.

Landslide Risk Assessment Tables

Table A – Risk to Property
Table B – Risk to Life

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Table A - Preliminary Landslide Risk Assessment (Risk to Property)
Alpine Coaster, Thredbo NSW

Possible Hazards			Consequences (Note 2)	Assessed Likelihood	Risk (Note 1)	Risk Treatment and Comments
Failure Envisaged	**Failure Mode**	**Initiating Circumstances**				
A - Generic slump affecting coaster footing	Slide	Groundwater, steep slope, surcharge loading	Minor	Unlikely	Low	Follow footing recommendations provided in report 5920-G1
B - General translational side affecting coaster footing	Slide	Groundwater, steep slope, surcharge loading	Medium	Unlikely	Low	Follow footing recommendations provided in report 5920-G1

Notes:

1. The risk assessment addresses only the consequences to property from potential landslide events considered relevant to the subject site. Injury to persons or potential for fatality from land sliding is not assessed in this table (refer Table B). The risk assessment is based on a preliminary appraisal only, carried out by inspection. Further assessment or quantification of the assessed geotechnical risks for the subject property would require additional data and/or investigation.
2. The consequences are for a development that is designed to accomodate the potential landslide risk or has demonstrated adequate performance over many years.
3. Refer to report and associated figures for illustration of possible hazards / slope failure mechanisms.
4. Refer to attachments for definitions and explanations of terms used in the risk assessment.

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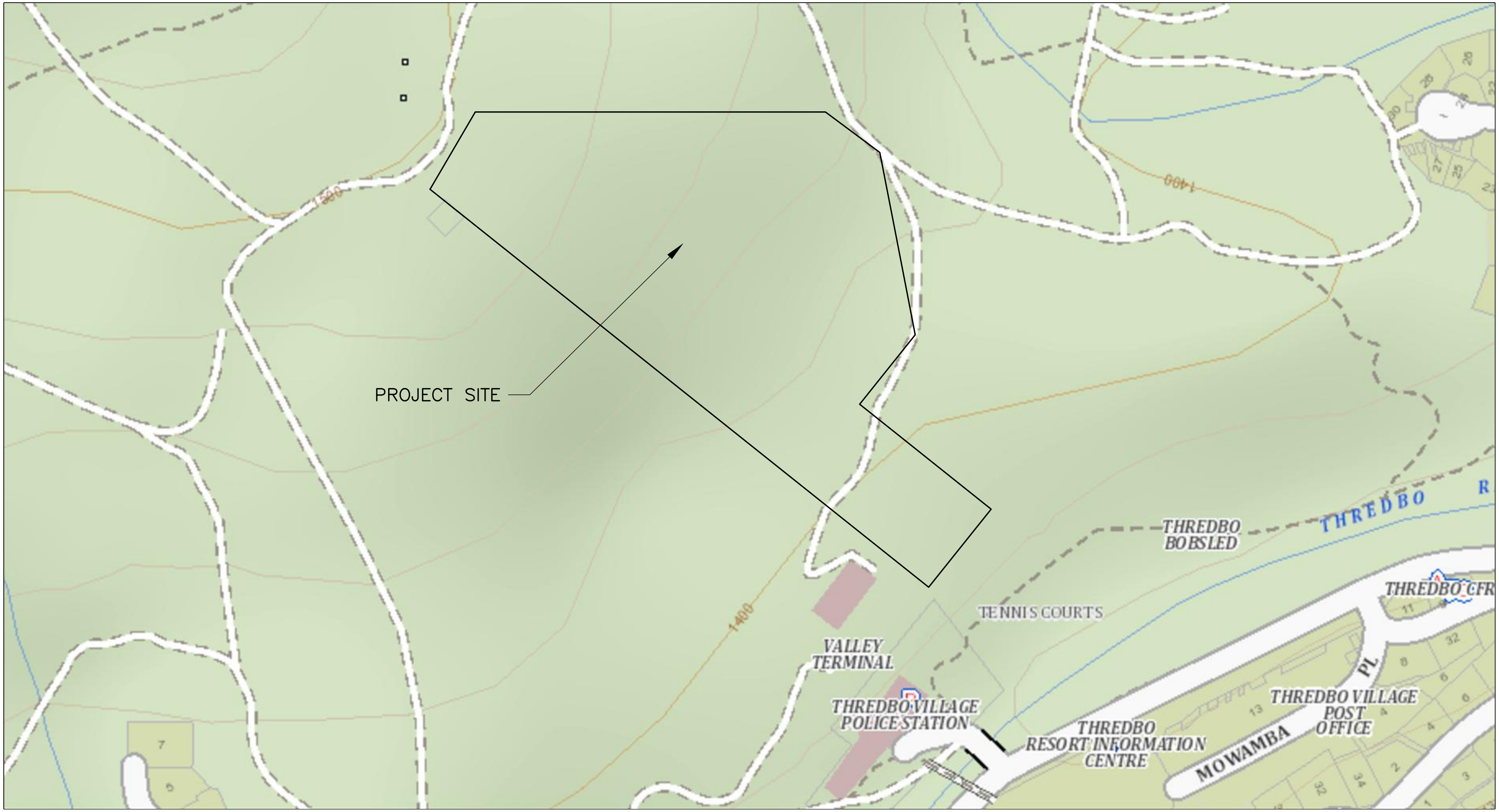
Table B - Preliminary Landslide Risk Assessment (Risk to Life)
Alpine Coaster, Thredbo NSW

Possible Hazard	Use of Affected Structure	Likelihood	Indicative Annual Probability P (H)	Probability of Spatial Impact P (S:H)	Temporal Probability P (T:S)	Vulner-ability V (D:T)	Probability of becoming Trapped	Risk for Person Most at Risk [Risk Evaluation]	Risk Outcome: A = Acceptable T = Tolerable NT = Not Tolerable
A - Generic slump affecting coaster footing	Occupants of coaster	Unlikely	1.0E-04	1.00	1.00	0.05	0.10	5.00E-07	A
B - General translational side affecting coaster footing	Occupants of coaster	Unlikely	1.0E-04	1.00	1.00	0.05	0.10	5.00E-07	A

Notes:
1. The appraisal of the assessed risk relative to acceptable and tolerable risks is based on Table 1 of AGS (2007) – Reference 1, for a new development.
2. Risk mitigation will be required to ensure that the assessed risk outcome during and after the proposed development is acceptable. Referred to report for further details.
3. This table must be read in conunction with Table A.
4. Risk Outcome:
   A = Acceptable ≤ 10-6
   T = Tolerable ≤ 10-5
   NT = Not Tolerable - treatment options to be assessed and implemented
5. Probability of spatial impact for coaster assuming car must pass by footing (or footings) that are affected by landslide (i.e. S:H = 1)
6. Temporal probability for coaster assuming car is passing by a footing (or footings) that is affected by landslide (i.e. T:S = 1)
7. Vulnerability assumes car design is adequate to protect life for design speeds and geometry.

Figures

Figure 1 – Site Locality
Figure 2 – Test Locations
Snowmaking Layout



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APPROXIMATE ONLY – SUBJECT TO DETAIL SURVEY.

SOURCE: SIX MAPS

THIS DRAWING IS USED TO ILLUSTRATE SITE LOCATION ONLY, AND MUST NOT BE USED FOR ANY OTHER PURPOSE.

issue	date	description
A	27.1.21	INITIAL ISSUE

assetgeoenviro
2.06/56 Delhi Rd
North Ryde NSW 2113
t: 02 9878 6005
e: info@assetgeoenviro.com.au

PROPOSED ALPINE COASTER,
THREDBO NSW
for
KT–EVT

SITE LOCALITY

drawn: AT
date: 27.1.2021
checked: MAB
scale: 1:3000

job no.: 5920
fig: 1
issue: A

APPROXIMATE ONLY – SUBJECT TO DETAIL SURVEY.
SOURCE: SITE ANALYSIS PLAN, PREPARED BY DJRD ARCHITECTS, JOB 20413, DWG NO A1.218, REV B, dated 14 APRIL 2022.
THIS DRAWING IS USED TO ILLUSTRATE TEST LOCATIONS ONLY, AND MUST NOT BE USED FOR ANY OTHER PURPOSE. COPYRIGHT OF SOURCE DRAWING REMAINS WITH DJRD ARCHITECTS.

LEGEND

△ HA/DCP

⌖ Test Pit

0 1:2,000 A3 10m

issue	date	description
B	15.5.22	Updated base plan
A	27.1.21	INITIAL ISSUE

assetgeoenviro
2.06/56 Delhi Rd
North Ryde NSW 2113
t: 02 9878 6005
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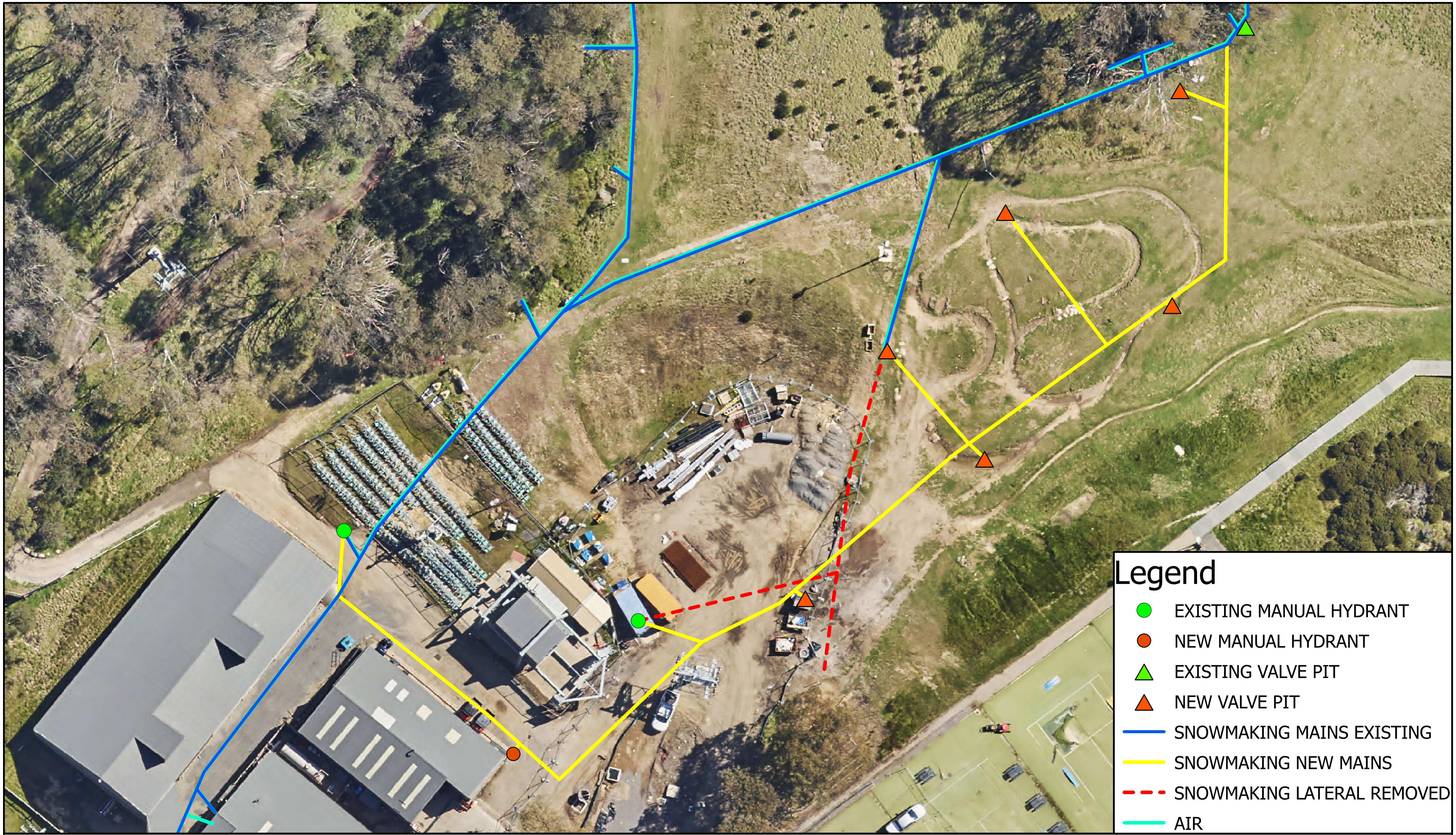
PROPOSED ALPINE COASTER, THREDBO NSW.
for
KT–EVT

TEST LOCATIONS

drawn: AT/MAB	job no.: 5920
date: 15.5.2022	
checked: MAB	fig: 2
scale: 1:2,000 A3	issue: B



Scale: 1:617

7 3.5 0 7 14 21 28 Meters

Map Projection: Universal Transverse Mercator
Horizontal Datum: GDA 2020
Grid: GDA 2020 MGA Zone 55

N

THREDBO

SNOWMAKING LAYOUT

Project: Snowmaking Services Proposal
Valley Terminal

Revision: G

Date: 26/07/2022

Produced By: KO

Appendix A

Important Information about your Geotechnical Report
Important Information about your Landslide Risk Assessment
GeoGuides (pp1-17)

Scope of Services

The geotechnical report ("the report") has been prepared in accordance with the scope of services as set out in the contract, or as otherwise agreed, between the Client and Asset Geotechnical Engineering Pty Ltd ("Asset"), for the specific site investigated. The scope of work may have been limited by a range of factors such as time, budget, access and/or site disturbance constraints.

The report should not be used if there have been changes to the project, without first consulting with Asset to assess if the report's recommendations are still valid. Asset does not accept responsibility for problems that occur due to project changes if they are not consulted.

Reliance on Data

Asset has relied on data provided by the Client and other individuals and organizations, to prepare the report. Such data may include surveys, analyses, designs, maps and plans. Asset has not verified the accuracy or completeness of the data except as stated in the report. To the extent that the statements, opinions, facts, information, conclusions and/or recommendations ("conclusions") are based in whole or part on the data, Asset will not be liable in relation to incorrect conclusions should any data, information or condition be incorrect or have been concealed, withheld, misrepresented or otherwise not fully disclosed to Asset.

Geotechnical Engineering

Geotechnical engineering is based extensively on judgment and opinion. It is far less exact than other engineering disciplines. Geotechnical engineering reports are prepared for a specific client, for a specific project and to meet specific needs, and may not be adequate for other clients or other purposes (e.g. a report prepared for a consulting civil engineer may not be adequate for a construction contractor). The report should not be used for other than its intended purpose without seeking additional geotechnical advice. Also, unless further geotechnical advice is obtained, the report cannot be used where the nature and/or details of the proposed development are changed.

Limitations of Site Investigation

The investigation program undertaken is a professional estimate of the scope of investigation required to provide a general profile of subsurface conditions. The data derived from the site investigation program and subsequent laboratory testing are extrapolated across the site to form an inferred geological model, and an engineering opinion is rendered about overall subsurface conditions and their likely behavior with regard to the proposed development. Despite investigation, the actual conditions at the site might differ from those inferred to exist, since no subsurface exploration program, no matter how comprehensive, can reveal all subsurface details and anomalies.

The engineering logs are the subjective interpretation of subsurface conditions at a particular location and time, made by trained personnel. The actual interface between materials may be more gradual or abrupt than a report indicates.

Therefore, the recommendations in the report can only be regarded as preliminary. Asset should be retained during the project implementation to assess if the report's recommendations are valid and whether or not changes should be considered as the project proceeds.

Subsurface Conditions are Time Dependent

Subsurface conditions can be modified by changing natural forces or man-made influences. The report is based on conditions that existed at the time of subsurface exploration. Construction operations adjacent to the site, and natural events such as floods, or ground water fluctuations, may also affect subsurface conditions, and thus the continuing adequacy of a geotechnical report. Asset should be kept appraised of any such events, and should be consulted to determine if any additional tests are necessary.

Verification of Site Conditions

Where ground conditions encountered at the site differ significantly from those anticipated in the report, either due to natural variability of subsurface conditions or construction activities, it is a condition of the report that Asset be notified of any variations and be provided with an opportunity to review the recommendations of this report. Recognition of change of soil and rock conditions requires experience and it is recommended that a suitably experienced geotechnical engineer be engaged to visit the site with sufficient frequency to detect if conditions have changed significantly.

Reproduction of Reports

This report is the subject of copyright and shall not be reproduced either totally or in part without the express permission of this Company. Where information from the accompanying report is to be included in contract documents or engineering specification for the project, the entire report should be included in order to minimize the likelihood of misinterpretation from logs.

Report for Benefit of Client

The report has been prepared for the benefit of the Client and no other party. Asset assumes no responsibility and will not be liable to any other person or organisation for or in relation to any matter dealt with or conclusions expressed in the report, or for any loss or damage suffered by any other person or organisation arising from matters dealt with or conclusions expressed in the report (including without limitation matters arising from any negligent act or omission of Asset or for any loss or damage suffered by any other party relying upon the matters dealt with or conclusions expressed in the report). Other parties should not rely upon the report or the accuracy or completeness of any conclusions and should make their own inquiries and obtain independent advice in relation to such matters.

Data Must Not Be Separated from The Report

The report as a whole presents the site assessment, and must not be copied in part or altered in any way.

Logs, figures, drawings, test results etc. included in our reports are developed by professionals based on their interpretation of field logs (assembled by field personnel) and laboratory evaluation of field samples. These data should not under any circumstances be redrawn for inclusion in other documents or separated from the report in any way.

Partial Use of Report

Where the recommendations of the report are only partially followed, there may be significant implications for the project and could lead to problems. Consult Asset if you are not intending to follow all of the report recommendations, to assess what the implications could be. Asset does not accept responsibility for problems that develop where the report recommendations have only been partially followed if they have not been consulted.

Other Limitations

Asset will not be liable to update or revise the report to take into account any events or emergent circumstances or fact occurring or becoming apparent after the date of the report.

Basis of The Assessment

Our assessment of landslide risk is presented in the framework of Landslide Risk Management (Australian Geomechanics Society, Vol 42, No 1, March 2007). The attached GeoGuides provide further information on landslide risk management and maintenance.

This assessment is based on a visual inspection of the property and the immediate adjoining land. Limited subsurface investigation may also have been undertaken as part of this appraisal. Slope monitoring has not been carried out within or adjacent to the property for the purpose of this appraisal. The opinions expressed in this report also consider our relevant local experience.

The property is within an area where landslip and/or subsidence have occurred, or where there is a risk of landslide. Important factors relating to slope conditions and the impact of development which commonly influence the landslide risks are discussed herein.

An owner's decision to acquire, develop or build on land within an area such as this involves the understanding and acceptance of a level of risk. It is important to recognise that soil and rock movements are an ongoing geological process, which may be affected by development and land management within the site or on ad- joining land. Soil and rock movements may cause visible damage to structures even where the risk of slope failure is considered low. This report is intended only to assess the landslide risk apparent at the time of inspection.

Our opinion is provided on the present landslide risk for the land specifically referenced in the title to this report. Foundations suitable for future building development are discussed in relation to slope stability considerations. Limited foundation advice may be provided. If so, advice is intended to guide the footing design for the proposed development. However, this report is not intended as, is not suitable for, and must not be used in lieu of a detailed foundation investigation for final design and costing of foundations, retaining walls or associated structures.

Limitations of The Assessment Procedure

The assessment procedures carried out for this appraisal are in accordance with the recommendations in Landslide Risk Management (Australian Geomechanics Society, Vol 42, No 1, March 2007), and with accepted local practice.

The following limitations must be acknowledged:

- the assessment of the stability of natural slopes requires a great degree of judgment and personal experience, even for experienced practitioners with good local knowledge;
- the assessment must be based on development of a sound geological model; slope processes and process rates influencing land sliding or landslide potential will vary according to geomorphologic influences;
- the likelihood that land sliding may occur on a given slope is generally hard to predict and is associated with significant uncertainties;
- different practitioners may produce different assessments of risk;
- actual risk of land sliding cannot be determined; risk changes with time;
- consequences of land sliding need to be considered in a rational framework of risk acceptance;
- acceptable risk in relation to damage to property from landslide activity is subjective; it remains the responsibility of the owner and/or local authority to decide whether the risk is acceptable; the geotechnical practitioner can assist with this judgment;
- the extent and methods of investigation for assessment of landslide risk will be governed by experience, by the perceived risk level, and by the degree to which the risk or consequences of land sliding are accepted for a specific project;
- the assessment may be required at several stages of the project or development; frequently (due to time or budget constraints imposed by the client) there will be no opportunity for long-term monitoring of the slope behaviour or groundwater conditions, or for on-going opportunity for the slope processes and performance of structures to be reviewed during and after development; such limitations should be recognised as relevant to the assessment.

Development on Slopes

Some landslide risk is always attached to the development of land on slopes.

Guidelines for hillside construction and examples of good practices for hillside developments are described in the attached GeoGuides.

THE AUSTRALIAN GEOGUIDES
FOR SLOPE MANAGEMENT AND MAINTENANCE

AGS Landslide Taskforce, Slope Management and Maintenance Working Group

The Australian Geomechanics Society (AGS) presents on the following pages a guideline on slope management and maintenance, as part of the landslide risk management guidelines developed under the National Disaster Funding Program (NDMP). This Guideline is aimed at home owners, developers and local councils, but also has applicability to a larger audience which includes builders and contractors, consultants, insurers, lawyers, government departments and in fact any person, or organisation, with a responsibility for the management or maintenance of a slope. The objective is to inform those with little or no knowledge of geotechnical engineering about landslides.

Each GeoGuide is a stand-alone document, which is formatted so that it can be printed on two sides of a single A4 sheet. It is expected that the set of GeoGuides will increase with time to cover a range of topics. As things stand:

- **GeoGuide LR1** is an introductory sheet that should be read by all users, since it explains what the LR (landslide risk) series is about and defines terms.
- **GeoGuides LR2, 3 and 4** explain why landslides occur and provide information on different types of landslide.
- **GeoGuide LR5** discusses the critical part that water often plays in relation to landslide occurrence and discusses measures that can be adopted to limit its effect.
- **GeoGuide LR6** refers to retaining walls and their maintenance.
- **GeoGuide LR7** puts the concept of landslide risk into an everyday context, so users can relate a particular landslide risk to other risks that they know they are prepared to take, sometimes on a daily basis.
- **GeoGuide LR8** retains the ideas of good and poor hillside construction practice originally provided by an AGS sub-committee in 1985.
- **GeoGuide LR9** concentrates specifically on effluent and surface water disposal, which is an important topic in some development areas.
- **GeoGuide LR10** is specifically aimed at those who have property on the coast and could be susceptible to coastal erosion processes.
- **GeoGuide LR11** provides information about the benefits of keeping records on inspection and maintenance activities and provides a proforma record sheet for users.

It is recognised that the GeoGuides are likely to be upgraded from time to time. Feedback on use and suggested changes should be sent to the National Chair of the Australian Geomechanics Society. The latest versions of the GeoGuides will be downloadable from the AGS website www.australiangemechanics.org

Through the NDMP, Australian governments (at Commonwealth, State and Local Government levels) are also funding the development of a Landslide Zoning Guideline (AGS 2007a), and a Practice Note Guideline (AGS 2007c) to which interested readers seeking in-depth information should refer.

ACKNOWLEDGEMENTS

These guidelines have been prepared by The Australian Geomechanics Society with funding from the National Disaster Mitigation Program, the Sydney Coastal Councils Group, and The Australian Geomechanics Society.

The Australian Geomechanics Society established a Working Group within a Landslide Taskforce to develop the guidelines. The development of the guidelines was managed by a Steering Committee. Membership of the Working Group, Taskforce and Steering Committee is listed in the Appendix.

Drafts of these GeoGuides have been subject to review by members of the AGS Landslide Taskforce, members of the geotechnical profession and local government.

REFERENCES

AGS (2007a) Guideline for Landslide Susceptibility, Hazard and Risk Zoning for Land Use Management. Australian Geomechanics Society, *Australian Geomechanics*, Vol 42, No1.

AGS (2007c). Practice Note Guidelines for Landslide Risk Management. Australian Geomechanics Society. *Australian Geomechanics*, Vol 42, No1,

AGS (2007e). The Australian GeoGuides for slope management and maintenance –. Australian Geomechanics Society. *Australian Geomechanics*, Vol 42, No 1, - this paper.

INTRODUCTION TO LANDSLIDE RISK

Photographs courtesy of Greg Kotze and Tony Phillips

AUSTRALIAN GEOGUIDES

The **Australian GeoGuides (LR series)** are a set of information sheets on the subject of landslide risk management and maintenance, published by the Australian Geomechanics Society (AGS). They provide background information intended to help people without specialist technical knowledge understand the basic issues involved. Topics covered include:

LR1 - Introduction	LR2 - Landslides	LR3 - Landslides in Soil
LR4 - Landslides in Rock	LR5 - Water & Drainage	LR6 - Retaining Walls
LR7 - Landslide Risk	LR8 - Hillside Construction	LR9 - Effluent & Surface Water Disposal
LR10 - Coastal Landslides	LR11 - Record Keeping	

The GeoGuides explain why slopes and retaining structures can be a hazard and what can be done with appropriate professional advice and local authority approval (if required) to remove, or reduce, the risk they represent.

Preparation of the GeoGuides has been funded by Australian governments through the National Disaster Mitigation Program (NDMP). This is a national program aimed at identifying and addressing natural disaster risk priorities across Australia. Technical input has been provided by experienced geotechnical engineers, engineering geologists and local government and government agency representatives from around Australia.

BACKGROUND

A number of landslides and cliff collapses occurred in Australia in the 1980's and 1990's in which lives were lost. Of these the Thredbo landslide probably received the most publicity, but there were several others. During this period the AGS issued a number of advisory notes to practitioners in relation to the assessment of landslide risk and its reduction. Building on these notes, and responding to changes in technology, a technical paper known as AGS2000 was prepared. It was followed in 2002 by an intensive nation-wide educational campaign attended by a large number of interested professionals from government departments and private industry. This resulted in an increased awareness of the risks associated with unstable slopes and a changed approach in many government departments responsible for regional planning, domestic development, roads, railways and the maintenance of natural features such as cliffs.

STATUS OF THE GEOGUIDES

The GeoGuides reflect the essence of good practice as perceived by a large number of geotechnical engineers, engineering geologists and other practitioners such as local government planners. **The GeoGuides are generic and do not, and cannot, constitute advice in relation to a specific situation. This must be sought from a geotechnical practitioner with first hand knowledge of the site.** It is expected that some local councils will refer to the GeoGuides and their companion publications in planning and building legislation. Check with your local council to see how it regards these documents. Companion publications to the GeoGuides are:

- AGS (2007a) Guideline for Landslide Susceptibility, Hazard and Risk Zoning for Land Use Management Australian Geomechanics Society, *Australian Geomechanics*, Vol 42, No1 and its associated commentary (AGS 2007b).
- AGS (2007c). Practice Note Guidelines for Landslide Risk Management. Australian Geomechanics Society. *Australian Geomechanics*, Vol 42, No1 2007, and its associated "Commentary" (AGS 2007d).

Copies of the above documents are available on the AGS website www.australiangeomechanics.org

TERMINOLOGY

Terminology tends to change with time and place and with the context in which it is used. The terms listed below have the following meanings in the GeoGuides:

Consequence	the outcome, or potential outcome, arising from the occurrence of a landslide expressed quantitatively, or qualitatively, in terms of loss, disadvantage, damage, injury, or loss of life.
Discontinuity	in relation to the ground is a crack, a bedding plane (a boundary between strata) or fault (a plane along which the ground has sheared) which forms a plane of weakness and reduces the overall strength of the ground.
Equilibrium	the condition when the forces on a mass of soil or rock in the ground, or on a retaining structure, are equal and opposite.
Factor of safety (FOS)	theoretically the forces available to prevent a part of the ground, or a retaining structure, from moving divided by those trying to move it. A FOS of one or less indicates that failure is likely to occur, but not how likely it is. To allow for unknowns and to limit movements engineers always aim to achieve a FOS significantly larger than one.
Failure	when part of the ground experiences movement as a result of the out of balance forces on it. Failure of a retaining structure means it is no longer able to fulfil its intended function.
Geotechnical practitioner	when referred to in the Australian GeoGuides (LR series), is a professional geotechnical engineer, or engineering geologist, with chartered status in a recognised national professional institution and relevant training, experience and core competencies in landslide risk assessment and management. In some government departments, technical officers are specifically trained to undertake some of the functions of a geotechnical practitioner.
Hazard	a condition with the potential for causing an undesirable consequence. In relation to landslides this includes the location, size, speed, distance of travel and the likelihood of its occurrence within a given period of time.
Landslide	the movement, or the potential movement, of a mass of rock, debris, or earth down a slope.
Likelihood	a qualitative description of probability, or frequency, of occurrence.
Partial saturation	the condition in the ground above the water table where both air and water are present as well as soil, or rock.
Perched water table	a water table above the true water table supported by a low permeability stratum.
Permeability	a measure of the ability of the ground to allow water to flow through it.
Risk	a measure of the probability and severity of an adverse effect to life, health, property or the environment.
Slip failure	landslide.
Stable	the condition when failure will not occur. Over geological time no part of the ground can be considered stable. Over short periods (eg the life of a structure) stability implies a very low likelihood of failure.
Retaining structure	anything built by humans which is intended to support the ground and inhibit failure.
Structure	in relation to rock, or soil, means the spacing, extent, orientation and type of discontinuities found in the ground at a particular location.
Tension crack	a distinct open crack that normally develops in the ground around a landslide and indicates actual, or imminent , failure.
Water table	the level in the ground below which it is saturated and the voids are filled with water.

LANDSLIDES

What is a Landslide?

Any movement of a mass of rock, debris, or earth, down a slope, constitutes a "landslide". Landslides take many forms, some of which are illustrated. More information can be obtained from Geoscience Australia, or by visiting its Australian Landslide Database at www.ga.gov.au/urban/factsheets/landslide.jsp. Aspects of the impact of landslides on buildings are dealt with in the book "Guideline Document Landslide Hazards" published by the Australian Building Codes Board and referenced in the Building Code of Australia. This document can be purchased over the internet at the Australian Building Codes Board's website www.abcb.gov.au .

Landslides vary in size. They can be small and localised or very large, sometimes extending for kilometres and involving millions of tonnes of soil or rock. It is important to realise that even a 1 cubic metre boulder of soil, or rock, weighs at least 2 tonnes. If it falls, or slides, it is large enough to kill a person, crush a car, or cause serious structural damage to a house. The material in a landslide may travel downhill well beyond the point where the failure first occurred, leaving destruction in its wake. It may also leave an unstable slope in the ground behind it, which has the potential to fail again, causing the landslide to extend (regress) uphill, or expand sideways. For all these reasons, both "potential" and "actual" landslides must be taken very seriously. They present a real threat to life and property and require proper management.

Identification of landslide risk is a complex task and must be undertaken by a geotechnical practitioner (GeoGuide LR1) with specialist experience in slope stability assessment and slope stabilisation.

What Causes a Landslide?

Landslides occur as a result of local geological and groundwater conditions, but can be exacerbated by inappropriate development (GeoGuide LR8), exceptional weather, earthquakes and other factors. Some slopes and cliffs never seem to change, but are actually on the verge of failing. Others, often moderate slopes (Table 1), move continuously, but so slowly that it is not apparent to a casual observer. In both cases, small changes in conditions can trigger a landslide with serious consequences. Wetting up of the ground (which may involve a rise in ground water table) is the single most important cause of landslides (GeoGuide LR5). This is why they often occur during, or soon after, heavy rain. Inappropriate development often results in small scale landslides which are very expensive in human terms because of the proximity of housing and people.

Does a Landslide Affect You?

Any slope, cliff, cutting, or fill embankment may be a hazard which has the potential to impact on people, property, roads and services. Some tell-tale signs that might indicate that a landslide is occurring are listed below:

- open cracks, or steps, along contours
- ground water seepage, or springs
- bulging in the lower part of the slope
- hummocky ground
- trees leaning down slope, or with exposed roots
- debris/fallen rocks at the foot of a cliff
- tilted power poles, or fences
- cracked or distorted structures

These indications of instability may be seen on almost any slope and are not necessarily confined to the steeper ones (Table 1). Advice should be sought from a geotechnical practitioner if any of them are observed. Landslides do not respect property boundaries. As mentioned above they can "run-out" from above, "regress" from below, or expand sideways, so a landslide hazard affecting your property may actually exist on someone else's land.

Local councils are usually aware of slope instability problems within their jurisdiction and often have specific development and maintenance requirements. **Your local council is the first place to make enquiries if you are responsible for any sort of development or own or occupy property on or near sloping land or a cliff.**

TABLE 1 - Slope Descriptions

Appearance	Slope Angle	Maximum Gradient	Slope Characteristics
Gentle	0°- 10°	1 on 6	Easy walking.
Moderate	10°- 18°	1 on 3	Walkable. Can drive and manoeuvre a car on driveway
Steep	18°- 27°	1 on 2	Walkable with effort. Possible to drive straight up or down roughened concrete driveway, but cannot practically manoeuvre a car.
Very Steep	27°- 45°	1 on 1	Can only climb slope by clutching at vegetation, rocks etc.
Extreme	45°- 64°	1 on 0.5	Need rope access to climb slope
Cliff	64°- 84°	1 on 0.1	Appears vertical. Can abseil down.
Vertical or Overhang	84°- 90±°	Infinite	Appears to overhang. Abseiler likely to lose contact with the face.

Some typical landslides which could affect residential housing are illustrated below:

Rotational or circular slip failures (Figure 1) - can occur on moderate to very steep soil and weathered rock slopes (Table 1). The sliding surface of the moving mass tends to be deep seated. Tension cracks may open at the top of the slope and bulging may occur at the toe. The ground may move in discrete "steps" separated by long periods without movement. More rapid movement may occur after heavy rain.

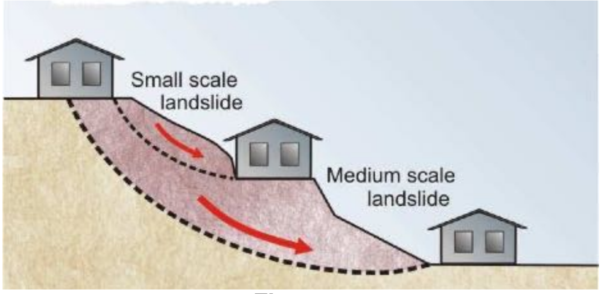


Figure 1

Translational slip failures (Figure 2) - tend to occur on moderate to very steep slopes (Table 1) where soil, or weak rock, overlies stronger strata. The sliding mass is often relatively shallow. It can move, or deform slowly (creep) over long periods of time. Extensive linear cracks and hummocks sometimes form along the contours. The sliding mass may accelerate after heavy rain.

Figure 2

Wedge failures (Figure 3) - normally only occur on extreme slopes, or cliffs (Table 1), where discontinuities in the rock are inclined steeply downwards out of the face.

Rock falls (Figure 3) - tend to occur from cliffs and overhangs (Table 1).

Cliffs may remain apparently unchanged for hundreds of years. Collections of boulders at the foot of a cliff may indicate that rock falls are ongoing. Wedge failures and rock falls do not "creep". Familiarity with a particular local situation can instil a false sense of security since failure, when it occurs, is usually sudden and catastrophic.

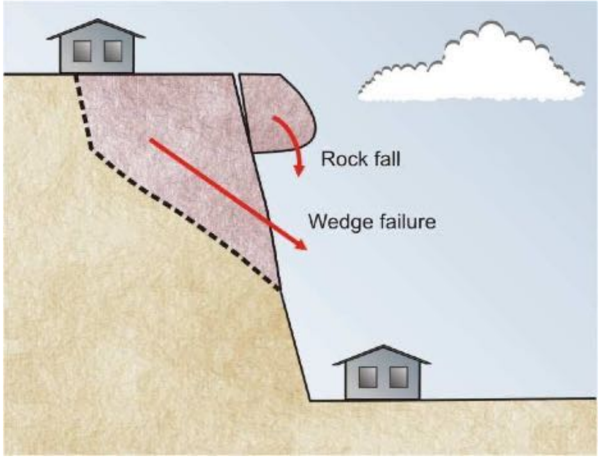


Figure 3

Debris flows and mud slides (Figure 4) - may occur in the foothills of ranges, where erosion has formed valleys which slope down to the plains below. The valley bottoms are often lined with loose eroded material (debris) which can "flow" if it becomes saturated during and after heavy rain. Debris flows are likely to occur with little warning; they travel a long way and often involve large volumes of soil. The consequences can be devastating.

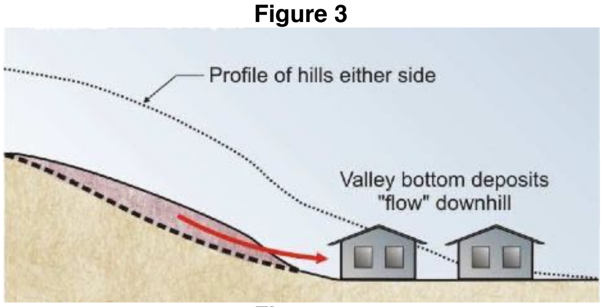


Figure 4

More information relevant to your particular situation may be found in other Australian GeoGuides:

- GeoGuide LR1 - Introduction
- GeoGuide LR3 - Soil Slopes
- GeoGuide LR4 - Rock Slopes
- GeoGuide LR5 - Water & Drainage
- GeoGuide LR6 - Retaining Walls
- GeoGuide LR7 - Landslide Risk
- GeoGuide LR8 - Hillside Construction
- GeoGuide LR9 - Effluent & Surface Water Disposal
- GeoGuide LR10 - Coastal Landslides
- GeoGuide LR11 - Record Keeping

The Australian GeoGuides (LR series) are a set of publications intended for property owners; local councils; planning authorities; developers; insurers; lawyers and, in fact, anyone who lives with, or has an interest in, a natural or engineered slope, a cutting, or an excavation. They are intended to help you understand why slopes and retaining structures can be a hazard and what can be done with appropriate professional advice and local council approval (if required) to remove, reduce, or minimise the risk they represent. The GeoGuides have been prepared by the Australian Geomechanics Society, a specialist technical society within Engineers Australia, the national peak body for all engineering disciplines in Australia, whose members are professional geotechnical engineers and engineering geologists with a particular interest in ground engineering. The GeoGuides have been funded under the Australian governments' National Disaster Mitigation Program.

AUSTRALIAN GEOGUIDE LR3 (LANDSLIDES IN SOIL)

LANDSLIDES IN SOIL

Landslides occur on soil slopes and the consequences can include damage to property and loss of life. Soil slopes exist in all parts of Australia and can even occur in places where rock outcrops can be seen on the surface. If you live on, or below, a soil slope it is important to understand why a landslide might occur and what you can do to reduce the risk it presents.

It is always worth asking the question *"why is this slope here?"*, because the answer often leads to an understanding of what might happen in the future. Slopes are usually formed by weathering (breakdown) and erosion (physical movement) of the natural ground - the "parent material". Many factors are involved including rain, wind, chemical change, temperature variation, plant growth, animal activity and our own human enthusiasm for development. The general process is outlined in Figure 1.

The upper levels of the parent material progressively weather over thousands, or millions, of years, losing strength. This can result in a surface layer which looks similar to the parent material (although its colour has probably changed) but has the strength of a soil - this is called "residual soil". At some stage the weathered surface layer is exposed to the elements and fragments are transported down the slope. In this context a fragment could be a single sand grain, a boulder, or a landslide. The time scale could be anything from a few seconds to many thousands of years. The transported fragments often collect on the lower slopes and form a new soil layer that blankets the original slope - "colluvium". If material reaches a river or the sea it is deposited as "alluvium" or as a "marine deposit". With appropriate changes in river and sea level this material can again find itself on the surface to commence another cycle of weathering and erosion. In places often, but not only, near the coast, this can include sand sized fragments which form beaches and are sometimes blown back onto the land to form dunes.

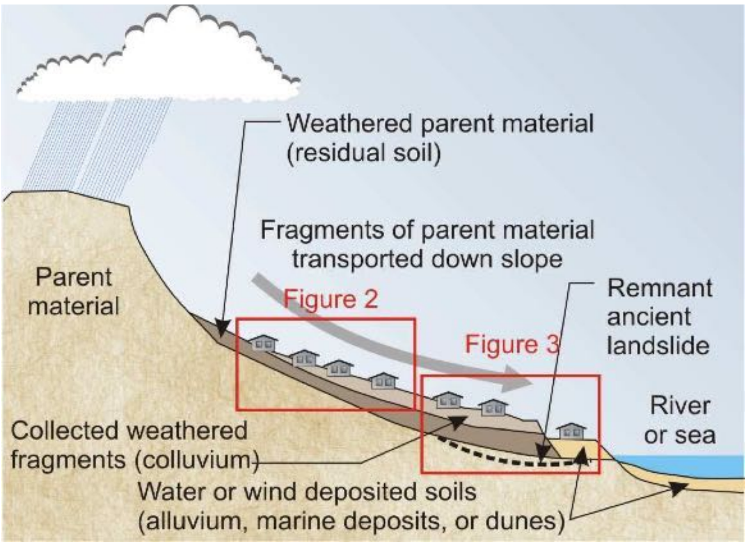


Figure 1

Landslides can occur almost anywhere on a soil slope. Slides can be rotational, translational, or debris flows (see GeoGuide LR2) and may have a number of causes.

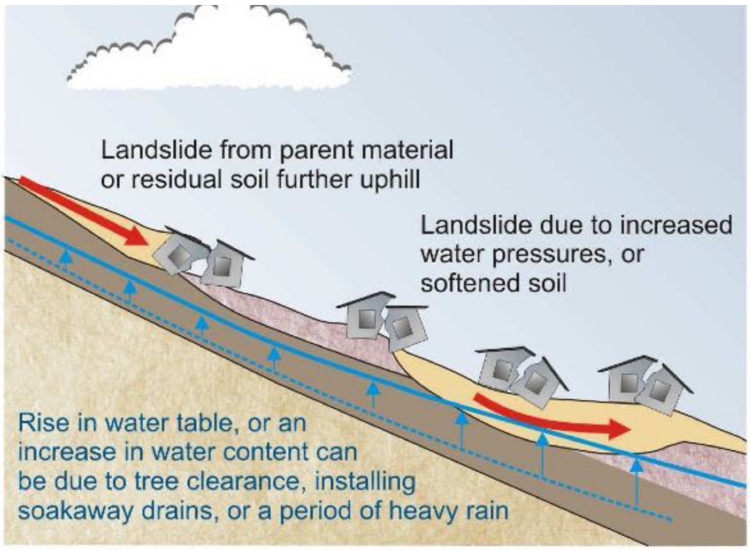


Figure 2

AUSTRALIAN GEOGUIDE LR3 (LANDSLIDES IN SOIL)

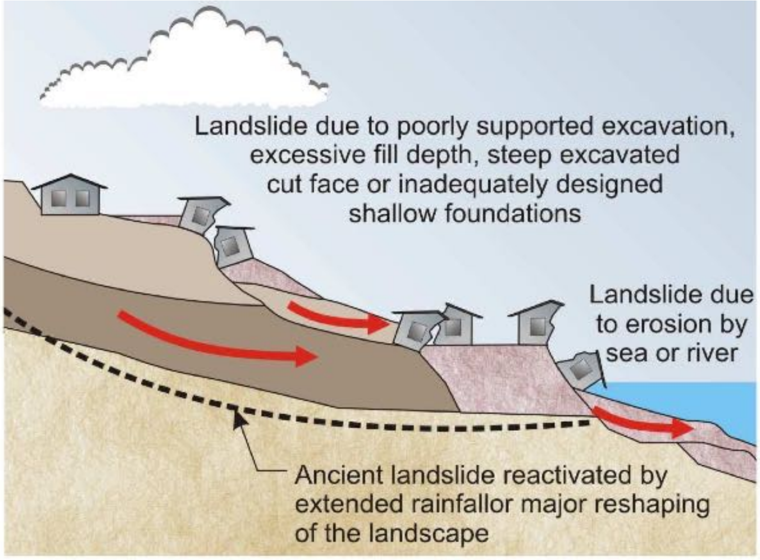


Figure 3

Some of the more common causes of landslides in soil are:

1) Falls of the parent material or residual soil from above, due to natural weathering processes (Figure 2).
2) Increased moisture content and consequent softening of the soil, or a rise in the water table. These can be due to excessive tree clearance, ill-considered soak-away drainage or septic systems, or heavy rainfall (Figure 2).
3) Excavation without adequate support, increased surface load from fill placement, or inadequately designed shallow foundations (Figure 3).
4) Natural erosion at the toe of the slope due to scour by a river or the sea (Figure 3).
5) Re-activation of an ancient landslide (Figure 3).

Most soil slopes appear stable, but they all achieved their present shape through a process of weathering and erosion and are often sensitive to minor changes in the factors that affect their stability. As a general rule, human activities only improve the situation if they have been designed to do so. Once this idea is understood, it is probably easy to see why the following basic rules are so important and should not be ignored without seeking site specific advice from a geotechnical practitioner:

- Do not clear trees unnecessarily.
- Do not cut into a slope without supporting the excavated face with an engineer designed structure.
- Do not add weight to a slope by placing earth fill or constructing buildings with inadequately designed shallow foundations (Note: in certain circumstances weight is added to the toe of a slope to inhibit landslide movement, but this must be carried out in accordance with a proper engineering design).
- Do not allow water from storm water drains, or from septic waste or effluent disposal systems to soak into the ground where it could trigger a landslide.

More information in relation to good and poor hillside construction practice is given in GeoGuide LR8. With appropriate engineering input it is often possible to reduce the likelihood, or consequences, of a landslide and so reduce the risk to property and to life. Such measures can include the construction of properly designed storm water and sub-soil drains, surface protection (GeoGuide LR5) and retaining walls (GeoGuide LR6). **Design should be undertaken by a geotechnical practitioner and will normally require local council approval.**

More information relevant to your particular situation may be found in other Australian GeoGuides:

- GeoGuide LR1 - Introduction
- GeoGuide LR2 - Landslides
- GeoGuide LR4 - Landslides in Rock
- GeoGuide LR5 - Water & Drainage
- GeoGuide LR6 - Retaining Walls
- GeoGuide LR7 - Landslide Risk
- GeoGuide LR8 - Hillside Construction
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- GeoGuide LR10 - Coastal Landslides
- GeoGuide LR11 - Record Keeping

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LANDSLIDES IN ROCK

Rocks have been formed by many different geological processes and may have been subjected to intense pressure, large scale distortion, extreme temperature and chemical change. As a result there are many different rock types and their condition varies enormously. Rock strength varies and is often significantly reduced by the presence of discontinuities (GeoGuide LR1). You may think that rock lasts forever, but in reality it weathers under the combined effects of water, wind, chemical change, temperature variation, plant growth and animal activity and erodes with time. Rock is often the parent material that ends up forming soil slopes (GeoGuide LR3). Inevitably different rocks have different physical and chemical characteristics and they weather and erode to form different types of soil.

Weathering can lead to landslides (GeoGuide LR2) on rock slopes. The type of landslide depends on the nature of rock, the way it has weathered and the presence or absence of discontinuities. It is hard to generalise, though normally a specific combination of discontinuities and material types will be the determining factor and these are often underground and out of sight. Typical examples are provided in the figures 1 to 4. A geotechnical practitioner can assess the landslide risk and propose appropriate maintenance measures. This often entails making geological observations over an area significantly larger than the site and a review of available background information, including records of known landslides and aerial photographs. Depending on the amount of information available, geotechnical investigation may or may not be needed. Every site is different and every site has to be assessed individually.

It is impossible to predict exactly when a landslide will occur on a rock slope, but failure is normally sudden and the consequences can be catastrophic.

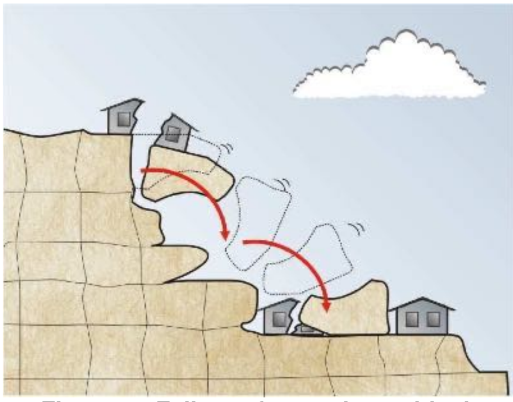

Figure 1 - Failure of an undercut block

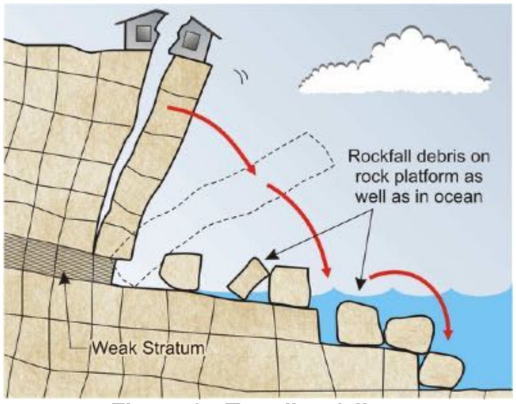

Figure 2 - Toppling failure

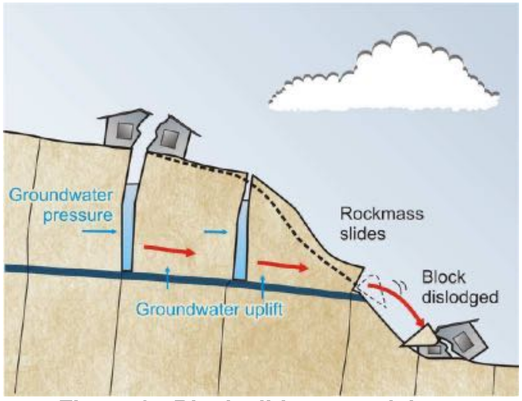

Figure 3 - Block slide on weak layer

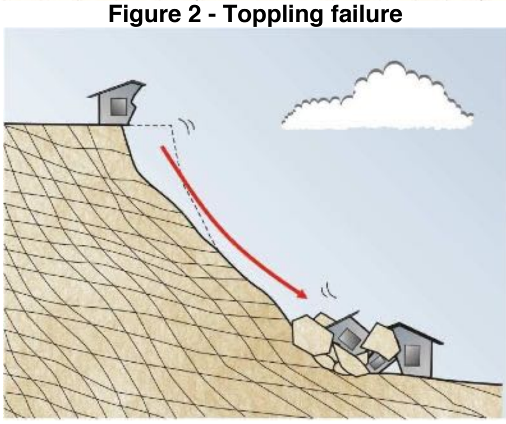

Figure 4 - Wedge failure along discontinuities

If the landslide risk is assessed as being anything other that Low, or Very Low, (GeoGuide LR7) it may be possible to carry out work aimed at reducing the level of risk.

The most common options are:

1) Trimming the slope to remove hazardous blocks of rock.
2) Bolting, or anchoring, to fix hazardous blocks in position and prevent movement.
3) Installation of catch fences and other rockfall protection measures to limit the impact of rockfalls.
4) Deep drainage designed to limit changes in the ground water table (GeoGuide LR5).

Although such measures can be effective, they need inspection and on-going maintenance (GeoGuide LR11) if they are to be effective for periods equivalent to the life of a house. **Design should be undertaken by a geotechnical practitioner and will normally require local council approval.** It should be appreciated that it may not be viable to carry out remedial works in all circumstances: for example where the landslide is on someone else's property, where the cost is out of proportion to the value of the property, or where the risk inherent in carrying out the work is actually greater than the risk of leaving things as they are. In situations such as these, development may be considered inappropriate.

ROCK SLOPE HAZARD REDUCTION MEASURES

Removal of loose blocks - may be effective but, depending on rock type, ongoing erosion can result in more blocks becoming unstable within a matter of years. Routine inspection, every 5 or so years, may be required to detect this.

Rock bolts and rock anchors (Figure 5) - can be installed in the ground to improve its strength and prevent individual blocks from falling. Rock bolts are usually tightened using a torque wrench, whilst rock anchors carry higher loads and require jacking. Both can be designed to be "permanent" using stainless steel, or sheathing, to inhibit corrosion, but the cost can be up to 10 times that of the "temporary" alternative. You should inspect rock bolts and rock anchors for signs of water seepage, rusting and deterioration around the heads at least once every 5 years. If you notice any of these warning signs, have them checked by a geotechnical practitioner. It is recommended that you keep copies of design drawings and maintenance records (GeoGuide LR11) for the anchors on your site and pass them on to the new owner should you sell.

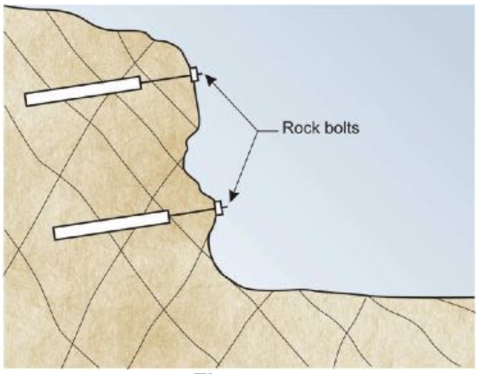

Figure 5

Rock fall netting, catch fences and catch pits (Figure 6) - are designed to catch or control falling rocks and prevent them from damaging nearby property. You should inspect them at least once every 5 years, and after major falls, and arrange for fallen and trapped rocks to be removed if they appear to be filling up. Check for signs of corrosion and replace steel elements and fixings before they lose significant strength.

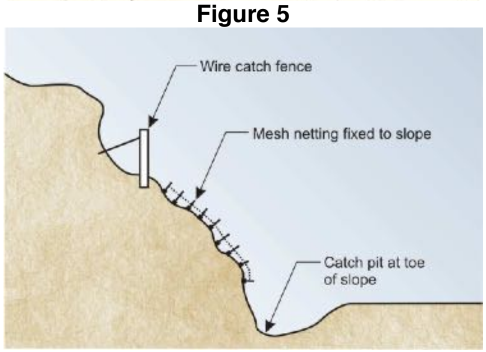

Figure 6

Cut-off drains (Figure 7) - can be used to intercept surface water run-off and reduce flows down the cliff face. Suitable drains are often excavated into the rock, or constructed from mounds of concrete, or stabilised soil, depending on conditions. Drains must be laid to a fall of at least 1% so they drain adequately. Frequent inspection is needed to ensure they are not blocked and continue to function as intended.

Clear trees and large bushes (Figure 7) - from slopes since roots can prize boulders from the face increasing the landslide hazard.

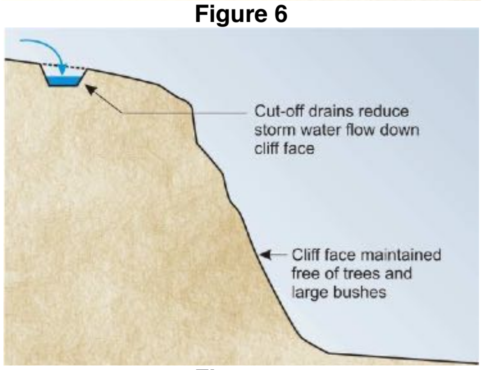

Figure 7

Natural cliffs and bluffs - often present the greatest hazard and yet are easily overlooked, because they have "been there forever". They can exist above a building, road, or beach, presenting the risk of a rock falling onto whatever is below. They also sometimes support buildings with a fine view to the horizon. Cliffs should be observed frequently to ensure that they are not deteriorating. You may find it convenient to use binoculars to look for signs of exposed "fresh" rock on the face, where a recent fall has occurred, or to go to the foot of the cliff from time to time to see if debris is collecting. A thorough inspection of a cliff face is often a major task requiring the use of rope access methods and should only be undertaken by an appropriately qualified professional. If tension cracks are observed in the ground at the top of a cliff take immediate action, since they could indicate imminent failure. **If you have any concerns at all about the possibility of a rock fall seek advice from a geotechnical practitioner.**

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WATER, DRAINAGE & SURFACE PROTECTION

One way or another, water usually plays a critical part in initiating a landslide (GeoGuide LR2). For this reason, it is a key factor to be controlled on sites with more than a low landslide risk (GeoGuide LR7).

Groundwater and Groundwater Flow

The ground is permeable and water flows through it as illustrated in Figure 1. When rain falls on the ground, some of it runs along the surface ("surface water run-off") and some soaks in, becoming groundwater. Groundwater seeps downwards along any path it can find until it meets the water table: the local level below which the ground is saturated. If it reaches the water table, groundwater either comes to a halt in what is effectively underground storage, or it continues to flow downwards, often towards a spring where it can seep out and become surface water again. Above the water table the ground is said to be "partially saturated", because it contains both water and air. Suctions can develop in the partially saturated zone which have the effect of holding the ground together and reducing the risk of a landslide. Vegetation and trees in particular draw large quantities of water out of the ground on a daily basis from the partially saturated zone. This lowers the water table and increases suctions, both of which reduce the likelihood of a landslide occurring.

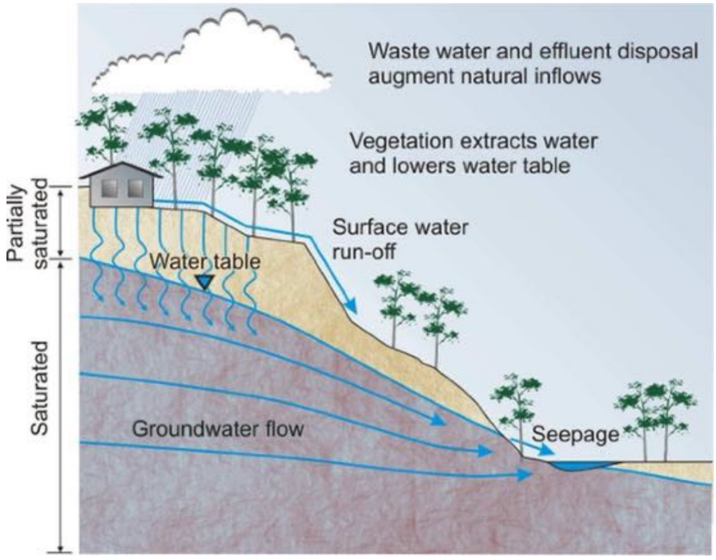

Figure 1 - Groundwater flow

Groundwater Flow and Landslides

The landslide risk in a hillside can be affected by increase in soak-away drainage or the construction of retaining walls which inhibit groundwater flow. The groundwater is likely to rise after heavy rain, but it can also rise when human interference upsets the delicate natural balance. Activities such as felling trees and earthworks can lead to:

- a reduction in the beneficial suctions in the partially saturated zone above the water table.
- increased static water pressures below the water table,
- increased hydraulic pressures due to groundwater flow,
- loss of strength, or softening, of clay rich strata,
- loss of natural cementing in some strata,
- transportation of soil particles.

Any of these effects, or a combination of them, can lead to landslides like those illustrated in GeoGuides LR2, LR3 and LR4.

Limiting the Effect of Water

Site clearance and construction must be carefully considered if changes in groundwater conditions are to be limited. GeoGuide LR8 considers good and poor development practices. Not surprisingly much of the advice relates to sensible treatment of water and is not repeated here. Adoption of appropriate techniques should make it possible to either maintain the current ground water table, or even cause it to drop, by limiting inflow to the ground.

If drainage measures and surface protection are relied on to keep the risk of a landslide to a tolerable level, it is important that they are inspected routinely and maintained (GeoGuide LR11).

The following techniques may be considered to limit the destabilising effects of rising groundwater due to development and are illustrated in Figure 2.

AUSTRALIAN GEOGUIDE LR5 (WATER & DRAINAGE)

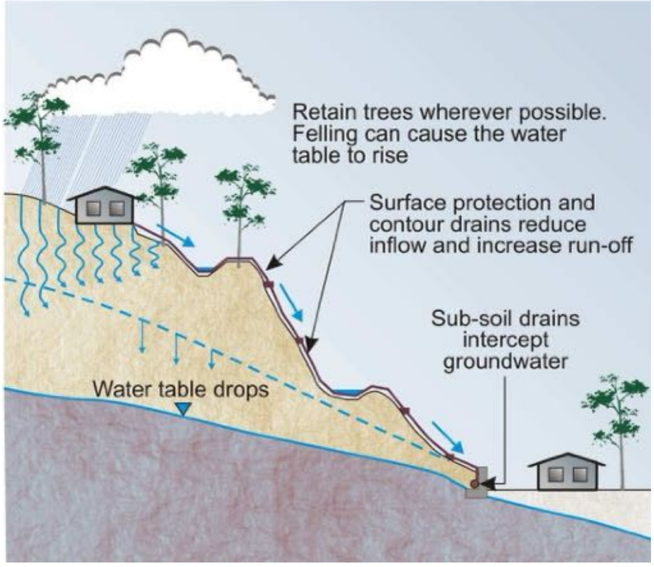

Figure 2 - Techniques used to control groundwater flow

Surface water drains (dish drains, or table drains) - are often used to prevent scour and limit inflow to a slope. Other than in rock, they are relatively ineffective unless they have an impermeable lining. You should clear them regularly, and as required, and not less than once a year. If you live in an area with seasonal rainfall, it is best to do this near the end of the dry season. If you notice that soil or rock debris is falling from the slope above, determine the source and take appropriate action. This may mean you have to seek advice from a geotechnical practitioner.

Surface protection - is sometimes used in addition to surface water drainage to prevent scour and minimise water inflow to a slope. You should inspect concrete, shotcrete or stone pitching for cracking and other signs of deterioration at least once a year. Make sure that weepholes are free of obstructions and able to drain. If the protection is deteriorating, you should seek advice from a geotechnical practitioner.

Sub-soil drains - are often constructed behind retaining walls and on hillsides to intercept groundwater. Their function is to remove water from the ground through an appropriate outlet. It is important that subsoil drains are designed to complement other measures being used. They should be laid in a sand, or gravel, bed and protected with a graded stone or geotextile filter to reduce the chance of clogging. Sub-soil drains should always be laid to a fall of at least 1 vertical on 100 horizontal. Ideally the high end should be brought to the surface, so it can be flushed with water from time to time as part of routine maintenance procedures.

Deep, underground drains - are usually only used in extreme circumstances, where the landslide risk is assessed as not being tolerable and other stabilisation measures are considered to be impractical. They work by permanently lowering the water table in a slope. They are not often used in domestic scale developments, but if you have any on your site be aware that professional maintenance is essential. If they are not maintained and stop working, the water table will rise and a landslide may even occur during normal weather conditions. Both an increase or a reduction in the normal flow from deep drains could indicate a problem if it appears to be unrelated to recent rainfall. If changes of this sort are observed, you should have the drains and your site checked by a geotechnical practitioner.

Documentation - design drawings and specifications for geotechnical measures intended to minimise landslide risk can be of great assistance to a geotechnical specialist, or structural engineer, called in to inspect and report on them. Copies of available documentation should be retained and passed to the new owner when the property is sold (GeoGuide LR11). You should also request details of an appropriate maintenance program for drainage works from the designer and keep that information with other relevant documentation and maintenance records.

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- GeoGuide LR3 - Landslides in Soil
- GeoGuide LR4 - Landslides in Rock
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RETAINING WALLS

Retaining walls are used to support cuts and fills. Some are built in the open and backfill is placed behind them (gravity walls). Others are inserted into the ground (cast *in situ* or driven piles) and the ground is subsequently excavated on one side. Retaining walls, like all man-made structures, have a finite life. Properly engineered walls should last 50 years, or more, without needing significant repairs. However, not all walls fit this category. Some, particularly those built by inexperienced tradesmen without engineering input, can deflect and even fail because they are unable to withstand the pressures that develop in the ground around them or because the materials from which they are built deteriorate with time. **Design of retaining walls more than 900mm high should be undertaken by a geotechnical practitioner or structural engineer and normally require local council approval.**

Retaining walls have to withstand the weight of the ground on the high side, any water pressure forces that develop, any additional load (surcharge) on the ground surface and sometimes swelling pressures from expansive clays. These forces are resisted by the wall itself and the ground on the low side. Engineers calculate the forces that the retained ground, the water, and the surcharge impose on a wall (the disturbing force) as well as the maximum force that the wall and ground on the low side can provide to resist them (the restoring force). The ratio of the restoring force to the disturbing force is called the "factor of safety" (GeoGuide LR1). Permanent retaining walls designed in accordance with accepted engineering standards will normally have a factor of safety in the range 1.5 to 2.

Never add surcharge to the high side of a wall (e.g. place fill, erect a structure, stockpile bulk materials, or park vehicles) unless you know the wall has been designed with that purpose in mind.

Never more than lightly water plants on the high side of a retaining wall.

Never excavate at the toe of a retaining wall.

Any of these actions will reduce the factor of safety of the wall and could lead to failure. If in doubt about any aspect of an existing retaining wall, or changes you would like to make near one, seek advice from a geotechnical practitioner, or a structural engineer. This GeoGuide sets out basic inspection requirements for retaining walls and identifies some common signs that might indicate all is not well. GeoGuide LR11 provides information about records that should be kept.

GRAVITY WALLS

Gravity walls are so called because they rely on their own weight (the force of gravity) to hold the ground behind in place.

Formed concrete and reinforced blockwork walls (Figure 1) **-** should be built so the backfill can drain. They should be inspected at least once a year. Look for signs of tilting, bulging, cracking, or a drop in ground level on the high side, as any of these may indicate that the wall has started to fail. Look for rust staining, which may indicate that the steel reinforcement is deteriorating and the wall is losing structural strength ("concrete cancer"). Ensure that weep holes are clear and that water is able to drain at all times, as high water pressures behind the wall can lead to sudden and catastrophic failure.

Concrete "crib" walls (Figure 2) **-** should be filled with clean gravel, or "blue metal" with a nominated grading. Sometimes soil is used to reduce cost, but this is undesirable, from an engineering perspective, unless internal drainage is incorporated in the wall's construction. Without backfill drainage, a soil filled crib wall is likely to have a lower factor of safety than is required. Crib walls should be inspected as for formed concrete walls. In addition, you should check that material is not being lost through the structure of the wall, which has large gaps through it.

Timber "crib" walls - should be checked as for concrete crib walls. In addition, check the condition of the timber. Once individual elements show signs of rotting, it is necessary to have the wall replaced. If you are uncertain seek advice from a geotechnical practitioner, or a structural engineer.

Masonry walls: natural stone, brick, or interlocking blocks (Figure 3) - more than about 1m high, should be wider at the bottom than at the top and include specific measures to permit drainage of the backfill. They should be checked as for formed concrete walls. Natural stone walls should be inspected for signs of deterioration of the individual blocks: strength loss, corners becoming rounded, cracks appearing, or debris from the blocks collecting at the foot of the wall.

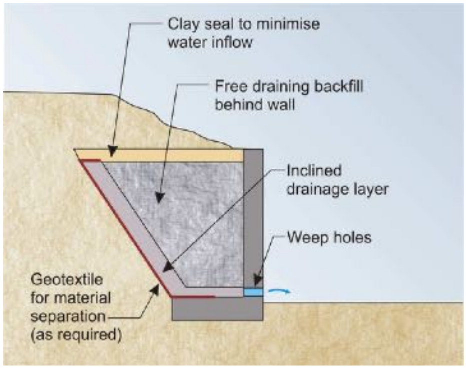

Figure 1- Typical formed concrete wall

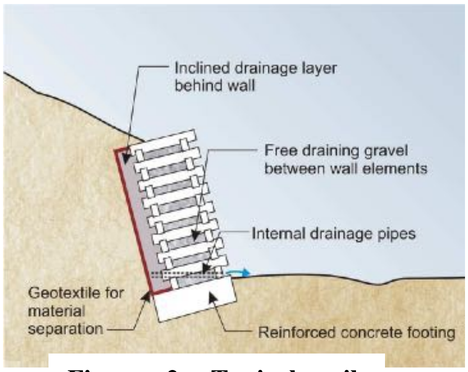

Figure 2 -Typical crib

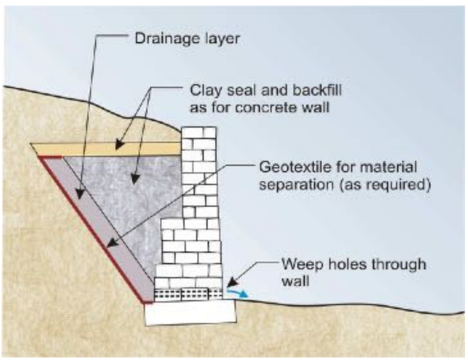

Figure 3 -Typical masonry wall

AUSTRALIAN GEOGUIDE LR6 (RETAINING WALLS)

Old Masonry walls (Figure 4) - Many old masonry retaining walls have not been built in accordance with modern design standards and often have a low "factor of safety" (GeoGuide LR1). They may therefore be close to failure and a minor change in their condition, or loading, could initiate collapse. You need to take particular care with such structures and seek professional advice sooner rather than later. Although masonry walls sometimes deflect significantly over long periods of time collapse, when it occurs, is usually sudden and can be catastrophic. Familiarity with a particular situation can instil a false sense of confidence.

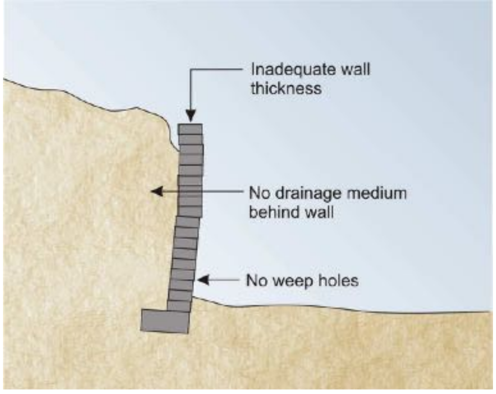

Figure 4 - Poorly built masonry wall

Reinforced soil walls (Figure 5) **-** are made of compacted select fill in which layers of reinforcement are buried to form a "reinforced soil zone". The reinforcement is all important, because it holds the soil "wall" together. Reinforcement may be steel strip, or mesh, or a variety of geosynthetic ("plastic") products. The facing panels are there to protect the soil "wall" from erosion and give it a finished appearance.

Most reinforced soil walls are proprietary products. Construction should be carried out strictly in accordance with the manufacturer's instructions. Inspection and maintenance should be the same as for formed concrete and concrete block walls. If unusual materials such as timber, or used tyres, are used as a facing it should be checked to see that it is not rotting, or perishing.

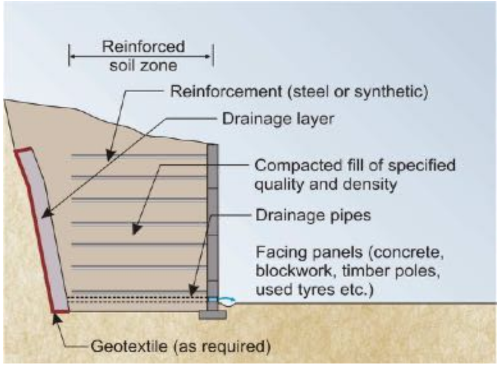

Figure 5 - Typical reinforced soil wall

OTHER WALLS

Cantilevered and anchored walls (Figure 6) **-** rely on earth pressure on the low side, rather than self-weight, to provided the restoring force and an adequate factor of safety. These walls may comprise:

- a line of touching bored piers (contiguous bored pile wall) or
- sprayed concrete panels between bored piers (shotcrete wall) or
- horizontal timber or concrete planks spanning between upright timber or steel soldier piles or
- steel sheet piles.

Depending on the form of construction and ground conditions, walls in excess of 3 m height normally require at least one row of permanent ground anchors.

INSPECTION

All walls should be inspected at least once a year, looking for tilting and other signs of deterioration. Concrete walls should be inspected for cracking and rust stains as for formed concrete gravity walls. Contiguous bored pile walls can have gaps between the piles - look for loss of soil from behind which can become a major difficulty if it is not corrected. Timber walls should be inspected for rot, as for timber crib walls. Steel sheet piles should be inspected for signs of rusting. In addition, you should make sure that ground anchors are maintained as described in GeoGuide LR4 under the heading "Rock bolts and rock anchors".

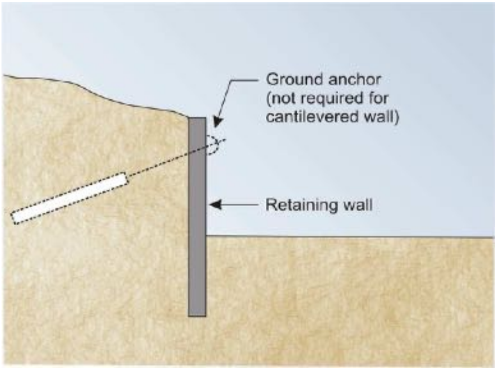

Figure 6 - Typical cantilevered or anchored wall

One of the most important issues for walls is that their internal drainage systems are operational. Frequently verify that internal drainage pipes and surface interception drains around the wall are not blocked nor have become inoperative.

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AUSTRALIAN GEOGUIDE LR7 (LANDSLIDE RISK)

LANDSLIDE RISK

Concept of Risk

Risk is a familiar term, but what does it really mean? It can be defined as *"a measure of the probability and severity of an adverse effect to health, property, or the environment."* This definition may seem a bit complicated. In relation to landslides, geotechnical practitioners (GeoGuide LR1) are required to assess risk in terms of the likelihood that a particular landslide will occur and the possible consequences. This is called landslide risk assessment. The consequences of a landslide are many and varied, but our concerns normally focus on loss of, or damage to, property and loss of life.

Landslide Risk Assessment

Some local councils in Australia are aware of the potential for landslides within their jurisdiction and have responded by designating specific "landslide hazard zones". Development in these areas is often covered by special regulations. If you are contemplating building, or buying an existing house, particularly in a hilly area, or near cliffs, go first for information to your local council.

Landslide risk assessment must be undertaken by a geotechnical practitioner. It may involve visual inspection, geological mapping, geotechnical investigation and monitoring to identify:

- potential landslides (there may be more than one that could impact on your site)
- the likelihood that they will occur
- the damage that could result
- the cost of disruption and repairs and
- the extent to which lives could be lost.

Risk assessment is a predictive exercise, but since the ground and the processes involved are complex, prediction tends to lack precision. If you commission a landslide risk assessment for a particular site you should expect to receive a report prepared in accordance with current professional guidelines and in a form that is acceptable to your local council, or planning authority.

Risk to Property

Table 1 indicates the terms used to describe risk to property. Each risk level depends on an assessment of how likely a landslide is to occur and its consequences in dollar terms. "Likelihood" is the chance of it happening in any one year, as indicated in Table 2. "Consequences" are related to the cost of repairs and temporary loss of use if a landslide occurs. These two factors are combined by the geotechnical practitioner to determine the Qualitative Risk.

TABLE 2: LIKELIHOOD

Likelihood	Annual Probability
Almost Certain	1:10
Likely	1:100
Possible	1:1,000
Unlikely	1:10,000
Rare	1:100,000
Barely credible	1:1,000,000

The terms "unacceptable", "may be tolerated", etc. in Table 1 indicate how most people react to an assessed risk level. However, some people will always be more prepared, or better able, to tolerate a higher risk level than others.

Some local councils and planning authorities stipulate a maximum tolerable level of risk to property for developments within their jurisdictions. In these situations the risk must be assessed by a geotechnical practitioner. If stabilisation works are needed to meet the stipulated requirements these will normally have to be carried out as part of the development, or consent will be withheld.

TABLE 1: RISK TO PROPERTY

Qualitative Risk		Significance - Geotechnical engineering requirements
Very high	VH	**Unacceptable** without treatment. Extensive detailed investigation and research, planning and implementation of treatment options essential to reduce risk to Low. May be too expensive and not practical. Work likely to cost more than the value of the property.
High	H	**Unacceptable** without treatment. Detailed investigation, planning and implementation of treatment options required to reduce risk to acceptable level. Work would cost a substantial sum in relation to the value of the property.
Moderate	M	**May be tolerated** in certain circumstances (subject to regulator's approval) but requires investigation, planning and implementation of treatment options to reduce the risk to Low. Treatment options to reduce to Low risk should be implemented as soon as possible.
Low	L	**Usually acceptable** to regulators. Where treatment has been needed to reduce the risk to this level, ongoing maintenance is required.
Very Low	VL	**Acceptable**. Manage by normal slope maintenance procedures.

Risk to Life

Most of us have some difficulty grappling with the concept of risk and deciding whether, or not, we are prepared to accept it. However, without doing any sort of analysis, or commissioning a report from an "expert", we all take risks every day. One of them is the risk of being killed in an accident. This is worth thinking about, because it tells us a lot about ourselves and can help to put an assessed risk into a meaningful context. By identifying activities that we either are, or are not, prepared to engage in we can get some indication of the maximum level of risk that we are prepared to take. This knowledge can help us to decide whether we really are able to accept a particular risk, or to tolerate a particular likelihood of loss, or damage, to our property (Table 2).

In Table 3, data from NSW for the years 1998 to 2002, and other sources, is presented. A risk of 1 in 100,000 means that, in any one year, 1 person is killed for every 100,000 people undertaking that particular activity. The NSW data assumes that the whole population undertakes the activity. That is, we are all at risk of being killed in a fire, or of choking on our food, but it is reasonable to assume that only people who go deep sea fishing run a risk of being killed while doing it.

It can be seen that the risks of dying as a result of falling, using a motor vehicle, or engaging in water-related activities (including bathing) are all greater than 1:100,000 and yet few people actively avoid situations where these risks are present. Some people are averse to flying and yet it represents a lower risk than choking to death on food. Importantly, the data also indicate that, even when the risk of dying as a consequence of a particular event is very small, it could still happen to any one of us any day. If this were not so, no one would ever be struck by lightning.

Most local councils and planning authorities that stipulate a tolerable risk to property also stipulate a tolerable risk to life. The AGS Practice Note Guideline recommends that 1:100,000 is tolerable in newly developed areas, where works can be carried out as part of the development to limit risk. The tolerable level is raised to 1:10,000 in established areas, where specific landslide hazards may have existed for many years. The distinction is deliberate and intended to prevent the concept of landslide risk management, for its own sake, becoming an unreasonable financial burden on existing communities. Acceptable risk is usually taken to be one tenth of the tolerable risk (1:1,000,000 for new developments and 1:100,000 for established areas) and efforts should be made to attain these where it is practicable and financially realistic to do so.

TABLE 3: RISK TO LIFE

Risk (deaths per participant per year)	**Activity/Event Leading to Death** (NSW data unless noted)
1:1,000	Deep sea fishing (UK)
1:1,000 to 1:10,000	Motor cycling, horse riding , ultra-light flying (Canada)
1:23,000	Motor vehicle use
1:30,000	Fall
1:70,000	Drowning
1:180,000	Fire/burn
1:660,000	Choking on food
1:1,000,000	Scheduled airlines (Canada)
1:2,300,000	Train travel
1:32,000,000	Lightning strike

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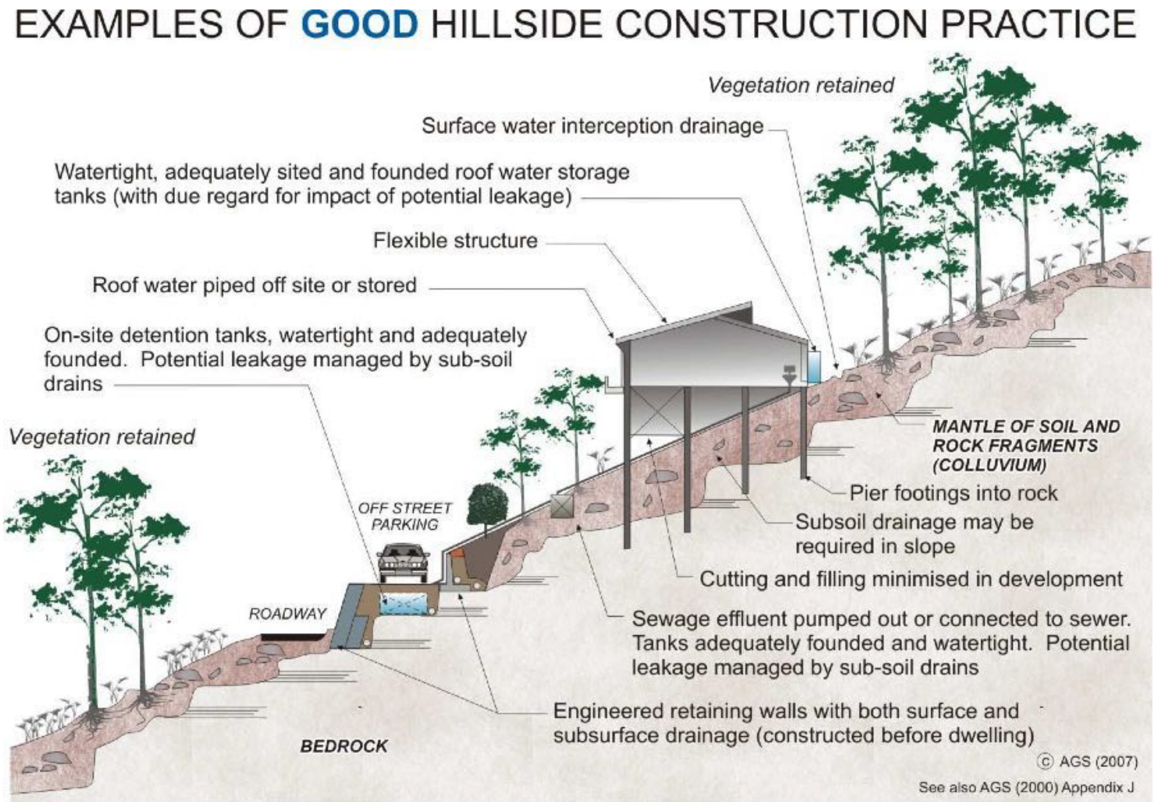
AUSTRALIAN GEOGUIDE LR8 (CONSTRUCTION PRACTICE)

HILLSIDE CONSTRUCTION PRACTICE

Sensible development practices are required when building on hillsides, particularly if the hillside has more than a low risk of instability (GeoGuide LR7). Only building techniques intended to maintain, or reduce, the overall level of landslide risk should be considered. Examples of good hillside construction practice are illustrated below.



WHY ARE THESE PRACTICES GOOD?

Roadways and parking areas - are paved and incorporate kerbs which prevent water discharging straight into the hillside (GeoGuide LR5).

Cuttings - are supported by retaining walls (GeoGuide LR6).

Retaining walls - are engineer designed to withstand the lateral earth pressures and surcharges expected, and include drains to prevent water pressures developing in the backfill. Where the ground slopes steeply down towards the high side of a retaining wall, the disturbing force (see GeoGuide LR6) can be two or more times that in level ground. Retaining walls must be designed taking these forces into account.

Sewage - whether treated or not is either taken away in pipes or contained in properly founded tanks so it cannot soak into the ground.

Surface water - from roofs and other hard surfaces is piped away to a suitable discharge point rather than being allowed to infiltrate into the ground. Preferably, the discharge point will be in a natural creek where ground water exits, rather than enters, the ground. Shallow, lined, drains on the surface can fulfil the same purpose (GeoGuide LR5).

Surface loads - are minimised. No fill embankments have been built. The house is a lightweight structure. Foundation loads have been taken down below the level at which a landslide is likely to occur and, preferably, to rock. This sort of construction is probably not applicable to soil slopes (GeoGuide LR3). If you are uncertain whether your site has rock near the surface, or is essentially a soil slope, you should engage a geotechnical practitioner to find out.

Flexible structures - have been used because they can tolerate a certain amount of movement with minimal signs of distress and maintain their functionality.

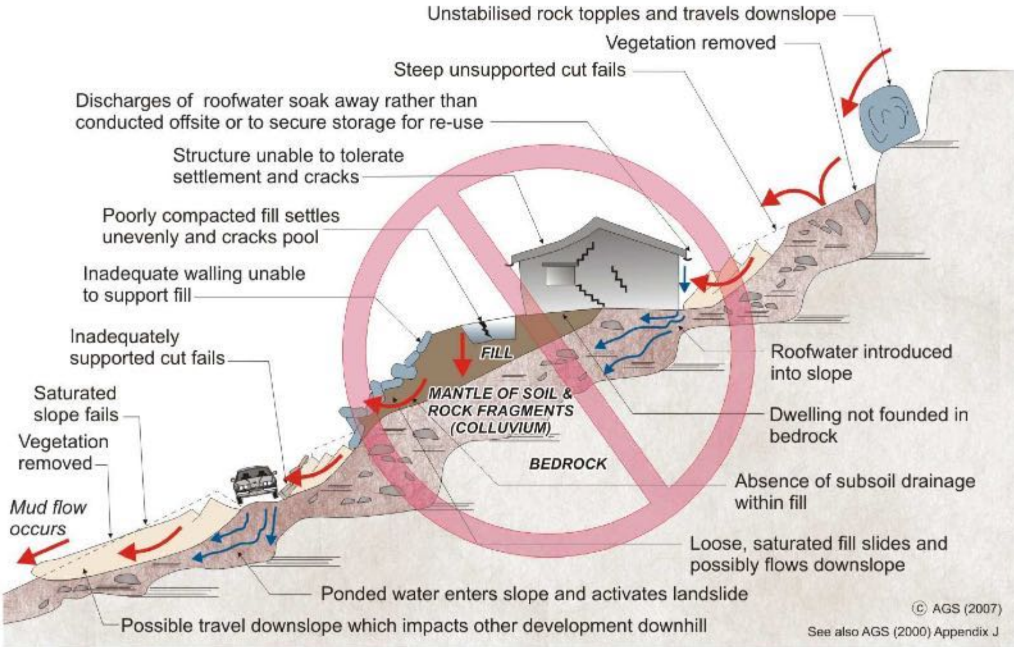
Vegetation clearance - on soil slopes has been kept to a reasonable minimum. Trees, and to a lesser extent smaller vegetation, take large quantities of water out of the ground every day. This lowers the ground water table, which in turn helps to maintain the stability of the slope. Large scale clearing can result in a rise in water table with a consequent increase in the likelihood of a landslide (GeoGuide LR5). An exception may have to be made to this rule on steep rock slopes where trees have little effect on the water table, but their roots pose a landslide hazard by dislodging boulders.

Possible effects of ignoring good construction practices are illustrated on page 2. Unfortunately, these poor construction practices are not as unusual as you might think and are often chosen because, on the face of it, they will save the developer, or owner, money. You should not lose sight of the fact that the cost and anguish associated with any one of the disasters illustrated, is likely to more than wipe out any apparent savings at the outset.

ADOPT GOOD PRACTICE ON HILLSIDE SITES

AUSTRALIAN GEOGUIDE LR8 (CONSTRUCTION PRACTICE)

EXAMPLES OF POOR HILLSIDE CONSTRUCTION PRACTICE



WHY ARE THESE PRACTICES POOR?

Roadways and parking areas - are unsurfaced and lack proper table drains (gutters) causing surface water to pond and soak into the ground.

Cut and fill - has been used to balance earthworks quantities and level the site leaving unstable cut faces and added large surface loads to the ground. Failure to compact the fill properly has led to settlement, which will probably continue for several years after completion. The house and pool have been built on the fill and have settled with it and cracked. Leakage from the cracked pool and the applied surface loads from the fill have combined to cause landslides.

Retaining walls - have been avoided, to minimise cost, and hand placed rock walls used instead. Without applying engineering design principles, the walls have failed to provide the required support to the ground and have failed, creating a very dangerous situation.

A heavy, rigid, house - has been built on shallow, conventional, footings. Not only has the brickwork cracked because of the resulting ground movements, but it has also become involved in a man-made landslide.

Soak-away drainage - has been used for sewage and surface water run-off from roofs and pavements. This water soaks into the ground and raises the water table (GeoGuide LR5). Subsoil drains that run along the contours should be avoided for the same reason. If felt necessary, subsoil drains should run steeply downhill in a chevron, or herring bone, pattern. This may conflict with the requirements for effluent and surface water disposal (GeoGuide LR9) and if so, you will need to seek professional advice.

Rock debris - from landslides higher up on the slope seems likely to pass through the site. Such locations are often referred to by geotechnical practitioners as "debris flow paths". Rock is normally even denser than ordinary fill, so even quite modest boulders are likely to weigh many tonnes and do a lot of damage once they start to roll. Boulders have been known to travel hundreds of metres downhill leaving behind a trail of destruction.

Vegetation - has been completely cleared, leading to a possible rise in the water table and increased landslide risk (GeoGuide LR5).

DON'T CUT CORNERS ON HILLSIDE SITES - OBTAIN ADVICE FROM A GEOTECHNICAL PRACTITIONER

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EFFLUENT AND SURFACE WATER DISPOSAL

EFFLUENT AND WASTEWATER

All households generate effluent and wastewater. The disposal of these products and their impact on the environment are key considerations in the planning of safe and sustainable communities. Cities and townships generally have reticulated water, sewer and stormwater systems, which are designed to deliver water and dispose of effluent and wastewater with minimal impact on the environment. However, many smaller communities and metropolitan fringe suburbs throughout Australia are un-sewered. Some of these are located in hillside or coastal settings where landslides present a hazard.

Processes by which wastewater can affect slope stability

As explained in GeoGuides LR3 and LR5, groundwater variations have a significant impact on slope stability. Inappropriate disposal of effluent and wastewater may result in the ground becoming saturated. The result is equivalent to a localised rise of the groundwater table and may have the potential to cause a landslide (GeoGuides LR2, LR5 and LR8).

On-site effluent disposal

In un-sewered areas disposal of effluent must be achieved through suitable methods. These methods usually involve containment within the boundaries of the site ("on-site disposal"). State environment protection agencies and local government authorities can usually provide advice on suitable disposal systems for your area. Such systems may include:

- *Septic systems,* which involve a storage/digestion tank for solids, with disposal of the liquid effluent via absorption trenches and beds, leach drains, or soak wells. Such systems are best suited to areas not prone to landslides.
- *Aerobic treatment units* which incorporate an individual household treatment plant to aid breakdown of the waste into a higher quality effluent. Such effluent is further treated and disposed of by surface or sub-surface irrigation, sub-soil dripper, or shallow leach drain system.
- *Nutrient retentive leaching systems* which utilise septic tanks to process the solid and liquid wastes in conjunction with discharge of the effluent through sand filters, media filters, mound systems and nutrient retentive leaching systems, which strip the effluent of nutrients.

Toilet (and sometimes kitchen) waste is known as *black water.* Other, less contaminated, wastewater streams from showers, baths and laundries are known as *grey water*. *Grey water re-use systems* allow a household to conserve water from bathrooms, kitchens and laundries, for re-use on gardens and lawns.

Recommendations for effluent disposal

In areas prone to landslide hazard, it is recommended that whatever effluent disposal system is employed, it should be designed by a qualified professional, familiar with how such a system can impact on the local environment. Local council, and in some instances state environment protection agency, approval is usually required as well. Many local authorities require a site assessment report, which covers all relevant issues. If approved, the report's recommendations must be incorporated in the system design. Reduction in the volume of effluent is beneficial so composting toilets and highly rated (i.e. low consumption) water appliances are recommended. It should be noted that in some state and local government jurisdictions there are restrictions on the alternative measures that can be applied. Consideration should be given to applying treated wastewater to land at low rates and over as large an area as possible. Further guidance can be found in Australian Standard AS/NZS 1547:2000 On-site domestic wastewater management.

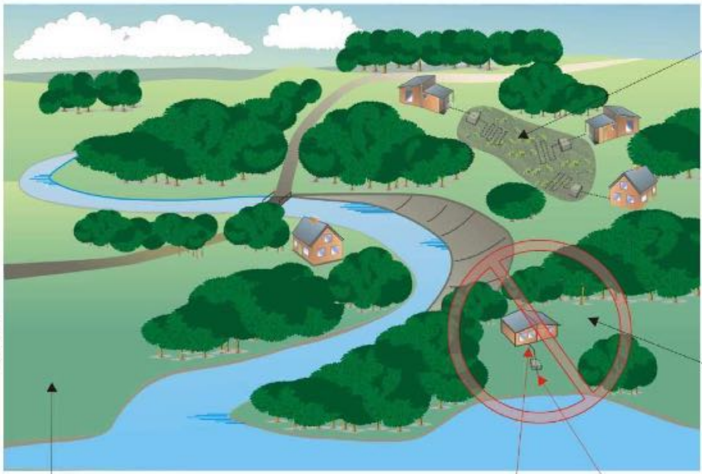
Effluent disposal fields should be sited with due consideration to the overall landscape and the individual characteristics of the property. Some guidance is provided. In particular, effluent fields should be located downslope of the building, away from stormwater, or *grey water*, discharge areas and where there is minimal potential for downstream pollution. Set backs and buffer distances vary from state to state and local requirements should be adhered to. All systems require regular maintenance and inspection. Efficient operation of the system must be a priority for property owners/occupiers to ensure safe and sustainable communities. Responsibility for maintenance rests with owners.

SURFACE WATER DRAINAGE

Attention to on-site surface water management is also important. Runoff from developments, including buildings, decks, access tracks and hardstand areas should be collected and discharged away from the development and other effluent disposal fields. Particular care must be given to the design of overflows on water tanks, as this is often overlooked. Discharge from any development should be spread out as much as possible, unless it can be directed to an existing natural water course. Ponding of water on hillsides and the concentration of water flows on slopes must be avoided.

It is recommended that a specific drainage plan and strategy should be developed in conjunction with the effluent disposal system for sites with a high potential for slope instability. Maintenance of the surface water drainage system is as important as maintenance of the effluent disposal system and again the responsibility rests with owners.

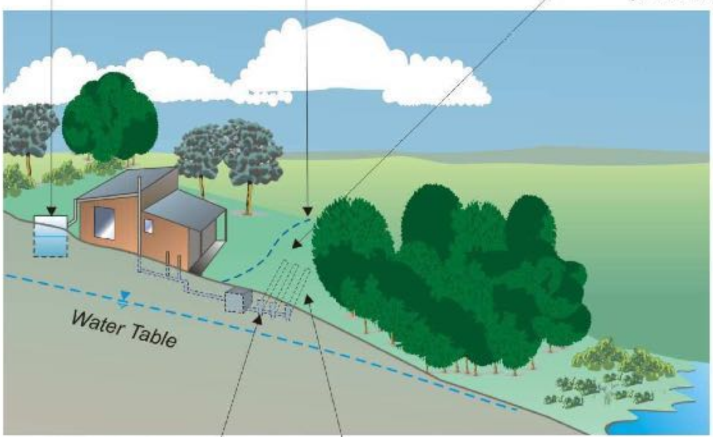
AUSTRALIAN GEOGUIDE LR9 (EFFLUENT DISPOSAL)



Reduce effluent volumes through highly rated appliances and grey water re-use systems

Avoid concentrations of surface water and direct away from effluent fields

Other effluent disposal systems can include soak wells, surface/spray irrigation, drip irrigation and subsurface drippers



Note: Adapted from EPA Vic. Publication 451 (March 1996) "Code of Practice - Septic Tanks", which was sourced from Vic. Department of Planning and Loddon-Campaspe Regional Planning Authority.

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LANDSLIDES IN THE COASTAL ENVIRONMENT

Coastal Instability

The coast presents a particularly dynamic environment where change is often the norm. Hazards exist in relation to both cliffs and sand dunes. The coast is also the most heavily populated part of Australia and always regarded as "prime" real estate, because of the views and access to waterways and beaches.

Photo courtesy Greg Kotze

Waves, wind and salt spray play a significant part, causing dunes to move and cliff-faces to erode well above sea level. Our response is often to try to neutralise these effects by doing such things as dumping rock in the sea, building groynes, dredging, or carrying out dune stabilisation. Such works can be very effective, but ongoing maintenance is usually needed and total reconstruction may be necessary after a relatively short working life.

Of particular significance are extreme events that cause destruction on a scale that ignores our efforts at coastal protection. Records show that cliffs have collapsed, taking with them backyards which had been relied upon as a buffer between a house and the ocean. Sand dunes have also been washed away resulting in the dramatic loss of homes and infrastructure. As with most landslide issues, even though such events may be infrequent, they could happen tomorrow. It is easy to be lulled into a false sense of security on a calm day.

In coastal areas, typical landslide hazards (GeoGuides LR1 to LR4) are compounded by coastal erosion which, over time, undercuts cliffs and eventually results in failure. In the case of sand dunes, dune erosion and dune slumping have equally dramatic effects. Coastal locations are subject to particular processes relating to fluctuating water tables, inundation under storm tides and direct wave attack. Large sections of our more sandy coastline are receding under present sea conditions. The hazards are progressive and likely to be exacerbated through climate change.

Coastal Development

If you own, or are responsible for, a coastal property it is important that you understand that, where the shore line is receding, there is a greater landslide risk than would be the case on a similar site inland. The view may make the risk worthwhile, but does not reduce it.

Coastal Landslides

Coastal landslides are little different from other landslides in that the signs of failure (GeoGuides LR2) and the causes (LR3, LR4 & LR5) are largely the same. The main difference relates to the overriding influence of wave impact, tidal movement, salt spray and high winds.

Cliff failures

In addition to the processes that produce cliff instability on inland cliffs, coastal cliffs are also subjected to repeated cycles of wetting and drying which can be accompanied by the expansive effect of salt crystal growth in gaps in the rocks. These processes accelerate the deterioration of coastal cliffs. At the base of cliffs, direct wave attack and the impact of boulders moved by wave action causes undercutting and hence instability of the overall face. Figure 2 of GeoGuide LR4 provides an example. Whilst the processes leading to coastal cliff collapse may take years, failure tends to be catastrophic and with little warning. In many cases, waves produced by large oceanic storms are the trigger assisted by rainfall to produce collapse. These are also the conditions in which you are more likely to be inside your home and oblivious to unusual noises or movements associated with imminent failure.

Sand dune escarpment and slope failures

An understanding of coastal processes is essential when determining beach erosion potential. Waves produced by large oceanic storms can erode beaches and cut escarpments into dunes. These may be of relatively short duration, when beach re-building happens after the storm, but can be a permanent feature where long term beach recession is taking place. In many locations, houses and infrastructure are sited on or immediately behind coastal dunes. After an escarpment has eroded, those assets may be lost or damaged by subsequent slumping of the dune. It is important that, on erodible coastal soils, the potential for landward incursion of an erosion escarpment is determined. Having done this, the likelihood of slope instability can be established as part of the landslide risk management process. Injury, death and structural damage have occurred around the Australian coast from collapsing sand escarpments.

Photo courtesy DNR NSW

AUSTRALIAN GEOGUIDE LR10 (COASTAL LANDSLIDES)

The large scale and potentially high speed of coastal erosion processes means that major civil engineering work and large cost is normally involved in their control. The installation of rock bolts (LR4), drainage (LR5), or retaining walls (LR6) on a single house site may be necessary to provide local stability, but are unlikely to withstand the attack of a large storm on a beach or cliff-line.

BUILDING NEAR CLIFFS AND HEADLANDS

Coastal cliffs and headlands exist because the rock that they are made from is able to resist erosion. Even so, cliff-faces are not immune and will continue to collapse (Figure 1) by one or other of the mechanisms shown on GeoGuide LR4. If you live on a coastal cliff, you should undertake inspection and maintenance as recommended in LR4 and the other GeoGuides, as appropriate. The top of the cliff, its face, and its base should be inspected frequently for signs of recent rock falls, opening of cracks, and heavy seepage which might indicate imminent failure. Since the sea can remove fallen rocks rapidly, inspections should be made shortly after every major storm as a matter of course. **If collapses are occurring seek advice from an appropriately experienced geotechnical practitioner. Advise you local council if you believe erosion is rapid or accelerating.**

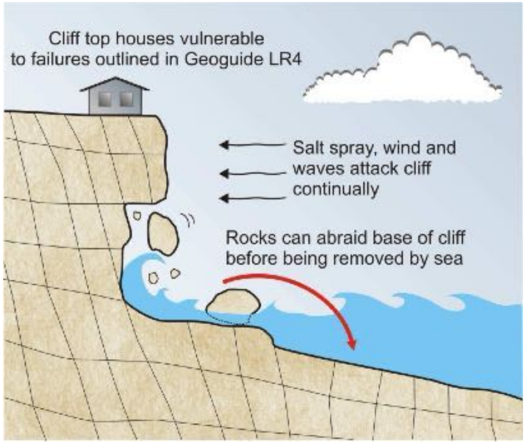

Figure 1

Building on Coastal Dunes

Any excavation in a natural dune slope is inherently unstable and must be supported and maintained (GeoGuide LR6). Dunes are particularly susceptible to ongoing erosion by wind and wave action and extreme changes can occur in a single storm. Whilst vegetation can help to stabilise dunes in the right circumstances, unfortunately a single storm has the potential to cut well into dunes and, in some cases, remove an entire low lying dune system or shift the mouth of a river. **As for cliffs, it is appropriate to observe the effects of major storms on the coastline. If erosion is causing the coastline to recede at an appreciable rate, seek advice from suitably experienced geotechnical and coastal engineering practitioners and bring it to the attention of the local council.**

Photo courtesy DNR NSW

CLIMATE CHANGE

The coastal zone will experience the most direct physical impacts of climate change. A number of reviews of global data indicate a general trend of sea level rise over the last century of 0.1 - 0.2 metres. Current rates of global average sea level rise, measured from satellite altimeter data over the last decade, exceed 3 mm/year and are accelerating. The most authoritative and recent (at the time of writing) report on climate change (IPCC, 2007) predicts a global average sea level rise of between 0.2 and 0.8 metres by 2100, compared with the 1980 - 1999 levels (the higher value includes the maximum allowance of 0.2 m to account for uncertainty associated with ice sheet dynamics).

In addition to sea level rise, climate change is also likely to result in changes in wave heights and direction, coastal wind strengths and rainfall intensity, all of which have the capacity to impact adversely on coastal dunes and cliff-faces. A Guideline for responding to the effects of climate change in coastal areas was published by Engineers Australia in 2004.

References

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IPCC (2007) *Climate Change 2007: The Physical Science Basis.* Summary for Policy Makers. Fourth Assessment Report of the Intergovernmental Panel on Climate Change (IPCC).

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AUSTRALIAN GEOGUIDE LR11 (RECORD KEEPING)

RECORD KEEPING

It is strongly recommended that records be kept of all construction, inspection and maintenance activities in relation to developments on sloping blocks. In some local authority jurisdictions, maintenance requirements form part of the building consent conditions, in which case they are mandatory.

CONSTRUCTION RECORDS

If at all possible, you should keep copies of drawings, specifications and construction (i.e. "as built") records, particularly if these differ from the design drawings. The importance of these documents cannot be over-emphasised. If a geotechnical practitioner comes to a site to carry out a landslide risk assessment and is only able to see the face of a retaining wall, the heads of some ground anchors, or the outlets of a number of sub-soil drains, it may be necessary to determine how these have been built and how they are meant to work before completing the assessment. This could involve drilling through the wall to determine how thick it is, or probing the length of the drains, or even ignoring the anchors altogether, because it is uncertain how long they are. Such "investigation" of something that may only have been built a few years before is, at best, a waste of time and money and, at worst, capable of coming up with a misleading answer which could affect the outcome of the assessment. Documentary information of this sort often proves to be invaluable later on, so treat it with as much importance as the title deeds to your property.

INSPECTION AND MAINTENANCE RECORDS

If you follow the recommendations of the Australian GeoGuides it is likely that you will either carry out periodic inspections yourself, or you will engage a geotechnical practitioner to do them for you. The collected records of these inspections will provide a detailed history of changes that might be occurring and will indicate, better than your own memory, whether things are deteriorating and, if so, at what rate. Unfortunately, without some form of written record, all information is usually lost each time a property is sold. It is recommended that a prospective purchaser should have a pre-purchase landslide risk assessment carried out on a hillside site, in much the same way that they would commission a structural assessment, or a pest inspection, of the building. If the vendor has kept good records, then the assessment is likely to be quicker and cheaper, and the outcome more reliable, than if none are available. Each site is different, but noting the following would normally constitute a reasonable record of an inspection/maintenance undertaken:

- date of inspection/maintenance and the name and professional status of the person carrying it out
- description of the specific feature (eg. cliff face, temporary rock bolt, cast *in situ* retaining wall, shallow leach drain system)
- sketch plans, sketches and photographs to indicate location and condition
- activity undertaken (eg. visual inspection; cleared vegetation from drain; removed fallen rock about 500 mm diameter)
- condition of the feature and any matters of concern (e.g. weep holes damp and flowing freely; rust on anchor heads getting worse; shotcrete uncracked and no sign of rust stains; ground saturated around leach field)
- specific outcomes (eg. no action necessary; geotechnical practitioner called in to advise on the state of the anchors; cliff face to be trimmed following the most recent rock fall; leach field to be rebuilt at new location)

A proforma record is provided overleaf for convenience. Photographs and sketches of specific observations can prove to be very useful and should be included whenever possible. Geotechnical practitioners may devise their own site specific inspection/maintenance records.

More information relevant to your particular situation may be found in other Australian GeoGuides:

- GeoGuide LR1 - Introduction
- GeoGuide LR2 - Landslides
- GeoGuide LR3 - Landslides in Soil
- GeoGuide LR4 - Landslides in Rock
- GeoGuide LR5 - Water & Drainage
- GeoGuide LR6 - Retaining Walls
- GeoGuide LR7 - Landslide Risk
- GeoGuide LR8 - Hillside Construction
- GeoGuide LR9 - Effluent & Surface Water Disposal
- GeoGuide LR10 - Coastal Landslides

The Australian GeoGuides (LR series) are a set of publications intended for property owners; local councils; planning authorities; developers; insurers; lawyers and, in fact, anyone who lives with, or has an interest in, a natural or engineered slope, a cutting, or an excavation. They are intended to help you understand why slopes and retaining structures can be a hazard and what can be done with appropriate professional advice and local council approval (if required) to remove, reduce, or minimise the risk they represent. The GeoGuides have been prepared by the Australian Geomechanics Society, a specialist technical society within Engineers Australia, the national peak body for all engineering disciplines in Australia, whose members are professional geotechnical engineers and engineering geologists with a particular interest in ground engineering. The GeoGuides have been funded under the Australian governments' National Disaster Mitigation Program.

INSPECTION/MAINTENANCE RECORD

(Tick boxes as appropriate and add information as required) Date..

Site location (street address / lot & DP numbers / map reference / latitude and longitude)

....

FEATURE

Feature				Inspected	Maintained	Tested	By Owner	By Professional
Slopes & surface protection:								
☐ Natural slope/cliff	☐ Cut/fill slope			☐	☐	☐	☐	☐
☐ Surface water drains				☐	☐	☐	☐	☐
☐ Shotcrete	☐ Stone pitching	☐ Other		☐	☐	☐	☐	☐
Retaining walls:								
☐ Cast in situ concrete	☐ Concrete block			☐	☐	☐	☐	☐
☐ Masonry (natural stone)	☐ Masonry (brick, block)			☐	☐	☐	☐	☐
☐ Cribwall (concrete)	☐ Cribwall (timber)			☐	☐	☐	☐	☐
☐ Anchored wall	☐ Reinforced soil wall			☐	☐	☐	☐	☐
☐ Sub-soil drains	☐ Weep holes			☐	☐	☐	☐	☐
Ground improvement:								
☐ Rock bolts				☐	☐	☐	☐	☐
☐ Ground anchors	☐ Soil nails			☐	☐	☐	☐	☐
☐ Deep subsoil drains				☐	☐	☐	☐	☐
Effluent and storm water disposal systems:								
☐ Effluent treatment system				☐	☐	☐	☐	☐
☐ Effluent disposal field				☐	☐	☐	☐	☐
☐ Storm water disposal field				☐	☐	☐	☐	☐
Other:								
☐ Netting	☐ Catch fence	☐ Catch pit		☐	☐	☐	☐	☐
☐ ------------------------------				☐	☐	☐	☐	☐
☐ ------------------------------				☐	☐	☐	☐	☐
☐ ------------------------------				☐	☐	☐	☐	☐

Observations/Notes (Add pages/details as appropriate)

....

....

....

Attachments: ☐ Sketch(es) ☐ Photograph(s) ☐ Other (eg measurements, test results)

Record prepared by .... (name): ....(signature)

Contact details: Phone:....................................... E-mail:..

Professional Status (in relation to landslide risk assessment):...

APPENDIX

AUSTRALIAN GEOMECHANICS SOCIETY

STEERING COMMITTEE

Andrew Leventhal, GHD Geotechnics, Sydney, Chair
Robin Fell, School of Civil and Environmental Engineering, UNSW, Sydney, Convenor Guidelines on Landslide Susceptibility, Hazard and Risk Working Group
Tony Phillips, Consultant, Sydney, Convenor Slope Management and Maintenance Working Group
Bruce Walker, Jeffery and Katauskas, Sydney, Convenor Practice Note Working Group
Geoff Withycombe, Sydney Coastal Councils Group, Sydney

WORKING GROUP - Guidelines on Slope Management and Maintenance
Tony Phillips, Tony Phillips Consulting, Sydney, Convenor
Henk Buys, NSW Roads and traffic Authority, Parramatta
John Braybrooke, Douglas Partners, Sydney
Tony Miner, A.G. Miner Geotechnical, Geelong

LANDSLIDE TASKFORCE
Laurie de Ambrosis, GHD Geotechnics, Sydney
Mark Eggers, Pells Sullivan Meynink, Sydney
Max Ervin, Golder Associates, Melbourne
Angus Gordon, retired, Sydney
Greg Kotze, GHD, Sydney
Arthur Love, Coffey Geotechnics, Newcastle
Alex Litwinowicz, GHD Geotechnics, Brisbane
Tony Miner, A.G. Miner Geotechnical, Geelong
Fiona MacGregor, Douglas Partners, Sydney
Garry Mostyn, Pells Sullivan Meynink, Sydney
Grant Murray, Sinclair Knight Merz, Auckland
Garth Powell, Coffey Geotechnics, Brisbane
Ralph Rallings, Pitt and Sherry, Hobart
Ian Stewart, NSW Roads and Traffic Authority, Sydney
Peter Tobin, Wollongong City Council, Wollongong
Graham Whitt, Shire of Yarra Ranges, Lillydale

Appendix B

Soil & Rock Explanation Sheets
Test Pit Logs
Borehole Logs
DCP Logs

Soil and Rock Explanation Sheets (1 of 2)

Log Abbreviations & Notes

METHOD

borehole logs		**excavation logs**	
AS	auger screw *	NE	natural excavation
AD	auger drill *	HE	hand excavation
RR	roller / tricone	BH	backhoe bucket
W	washbore	EX	excavator bucket
CT	cable tool	DZ	dozer blade
HA	hand auger	R	ripper tooth
D	diatube		
B	blade / blank bit		
V	V-bit		
T	TC-bit		

* bit shown by suffix e.g. ADV

coring
NMLC, NQ, PQ, HQ

SUPPORT

borehole logs		***excavation logs***	
N	nil	N	nil
M	mud	S	shoring
C	casing	B	benched
NQ	NQ rods		

CORE–LIFT

				casing installed
⊢⊣	barrel withdrawn

NOTES, SAMPLES, TESTS

D	disturbed
B	bulk disturbed
U50	thin-walled sample, 50mm diameter
HP	hand penetrometer (kPa)
SV	shear vane test (kPa)
DCP	dynamic cone penetrometer (blows per 100mm penetration)
SPT	standard penetration test
N*	SPT value (blows per 300mm) * denotes sample taken
Nc	SPT with solid cone
R	refusal of DCP or SPT

USCS SYMBOLS

GW	Gravel and gravel-sand mixtures, little or no fines.
GP	Gravel and gravel-sand mixtures, little or no fines, uniform gravels
GM	Gravel-silt mixtures and gravel-sand-silt mixtures.
GC	Gravel-clay mixtures and gravel-sand-clay mixtures.
SW	Sand and gravel-sand mixtures, little or no fines.
SP	Sand and gravel sand mixtures, little or no fines.
SM	Sand-silt mixtures.
SC	Sand-clay mixtures.
ML	Inorganic silt and very fine sand, rock flour, silty or clayey fine sand or silt with low plasticity.
CL, CI	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays.
OL	Organic silts
MH	Inorganic silts
CH	Inorganic clays of high plasticity.
OH	Organic clays of medium to high plasticity, organic silt
PT	Peat, highly organic soils.

MOISTURE CONDITION

D	dry
M	moist
W	wet
Wp	plastic limit
Wl	liquid limit

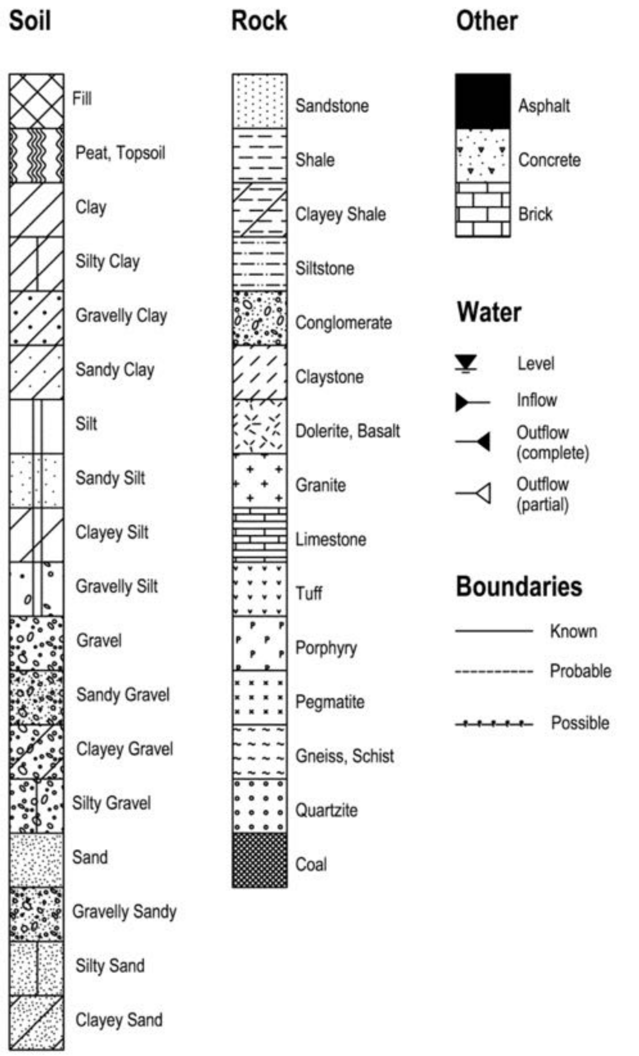
CONSISTENCY

VS	very soft
S	soft
F	firm
St	stiff
VSt	very stiff
H	hard
Fb	friable

DENSITY INDEX

VL	very loose
L	loose
MD	medium dense
D	dense
VD	very dense

Graphic Log



WEATHERING

XW	extremely weathered
HW	highly weathered
MW	moderately weathered
SW	slightly weathered
FR	fresh

STRENGTH

VL	very low
L	low
M	medium
H	high
VH	very high
EH	extremely high

RQD (%)

$$= \frac{\text{sum of intact core pieces > 2 x diameter}}{\text{total length of core run drilled}} \times 100$$

DEFECTS:

type		***coating***	
JT	joint	cl	clean
PT	parting	st	stained
SZ	shear zone	ve	veneer
SM	seam	co	coating

shape		***roughness***	
pl	planar	po	polished
cu	curved	sl	slickensided
un	undulating	sm	smooth
st	stepped	ro	rough
ir	irregular	vr	very rough

inclination
measured above axis and perpendicular to core

Soil and Rock Explanation Sheets (2 of 2)

AS1726-2017

Soils and rock are described in the following terms, which are broadly in accordance with AS1726-2017.

Soil

MOISTURE CONDITION

Term	Description
Dry	Looks and feels dry. Fine grained and cemented soils are hard, friable or powdery. Uncemented coarse grained soils run freely through hand.
Moist	Soil feels cool and darkened in colour. Fine grained soils can be moulded. Coarse soils tend to cohere.
Wet	As for moist, but with free water forming on hand.

Moisture content of cohesive soils may also be described in relation to plastic limit (W_P) or liquid limit (W_L) [>> much greater than, > greater than, < less than, << much less than].

CONSISTENCY OF FINE-GRAINED SOILS

Term	Su (kPa)	Term	Su (kPa)
Very soft	< 12	Very Stiff	>100 – ≤200
Soft	>12 – ≤25	Hard	> 200
Firm	>25 – ≤50	Friable	–
Stiff	>50 – ≤100		

RELATIVE DENSITY OF COARSE-GRAINED SOILS

Term	Density Index (%)	Term	Density Index (%)
Very Loose	< 15	Dense	65 – 85
Loose	15 – 35	Very Dense	>85
Medium Dense	35 – 65		

PARTICLE SIZE

Name	Subdivision	Size (mm)
Boulders		> 200
Cobbles		63 – 200
Gravel	coarse	19 – 63
	medium	6.7 – 19
	fine	2.36 – 6.7
Sand	coarse	0.6 – 2.36
	medium	0.21 – 0.6
	fine	0.075 – 0.21
Silt & Clay		< 0.075

MINOR COMPONENTS

Term	Proportion by Mass: *coarse grained*	*fine grained*
Trace	≤ 15%	≤ 5%
With	>15% – ≤30%	>5% – ≤12%

SOIL ZONING

Layers	Continuous across exposures or sample.
Lenses	Discontinuous, lenticular shaped zones.
Pockets	Irregular shape zones of different material.

SOIL CEMENTING

Weakly	Easily broken up by hand pressure in water or air.
Moderately	Effort is required to break up by hand in water or in air.

USCS SYMBOLS

Symbol	Description
GW	Gravel and gravel-sand mixtures, little or no fines.
GP	Gravel and gravel-sand mixtures, little or no fines, uniform gravels.
GM	Gravel-silt mixtures and gravel-sand-silt mixtures.
GC	Gravel-clay mixtures and gravel-sand-clay mixtures.
SW	Sand and gravel-sand mixtures, little or no fines.
SP	Sand and gravel sand mixtures, little or no fines.
SM	Sand-silt mixtures.
SC	Sand-clay mixtures.
ML	Inorganic silt and very fine sand, rock flour, silty or clayey fine sand or silt with low plasticity.
CL, CI	Inorganic clays of low to medium plasticity, gravelly clays, sandy clays.
OL	Organic silts
MH	Inorganic silts
CH	Inorganic clays of high plasticity.
OH	Organic clays of medium to high plasticity, organic silt
PT	Peat, highly organic soils.

Rock

SEDIMENTARY ROCK TYPE DEFINITIONS

Rock Type	Definition (more than 50% of rock consists of …..)
Conglomerate	... gravel sized (>2mm) fragments.
Sandstone	... sand sized (0.06 to 2mm) grains.
Siltstone	... silt sized (<0.06mm) particles, rock is not laminated.
Claystone	... clay, rock is not laminated.
Shale	... silt or clay sized particles, rock is laminated.

LAYERING

Term	Description
Massive	No layering apparent.
Poorly Developed	Layering just visible. Little effect on properties.
Well Developed	Layering distinct. Rock breaks more easily parallel to layering.

STRUCTURE

Term	Spacing (mm)	Term	Spacing
Thinly laminated	<6	Medium bedded	200 – 600
Laminated	6 – 20	Thickly bedded	600 – 2,000
Very thinly bedded	20 – 60	Very thickly bedded	> 2,000
Thinly bedded	60 – 200		

STRENGTH (NOTE: Is50 = Point Load Strength Index)

Term	Is50 (MPa)	Term	Is50 (MPa)
Extremely Low	<0.03	High	1.0 – 3.0
Very low	0.03 – 0.1	Very High	3.0 – 10.0
Low	0.1 – 0.3	Extremely High	>10.0
Medium	0.3 – 1.0		

WEATHERING

Term	Description
Residual Soil	Material is weathered to an extent that it has soil properties. Rock structures are no longer visible, but the soil has not been significantly transported.
Extremely …..	Material is weathered to the extent that it has soil properties. Mass structures, material texture & fabric of original rock is still visible.
Highly …..	Rock strength is significantly changed by weathering; rock is discolored, usually by iron staining or bleaching. Some primary minerals have weathered to clay minerals.
Moderately …..	Rock strength shows little or no change of strength from fresh rock; rock may be discolored.
Slightly …..	Rock is partially discolored but shows little or no change of strength from fresh rock.
Fresh	Rock shows no signs of decomposition or staining.

DEFECT DESCRIPTION

Type	
Joint	A surface or crack across which the rock has little or no tensile strength. May be open or closed.
Parting	A surface or crack across which the rock has little or no tensile strength. Parallel or sub-parallel to layering/bedding. May be open or closed.
Sheared Zone	Zone of rock substance with roughly parallel, near planar, curved or undulating boundaries cut by closely spaced joints, sheared surfaces or other defects.
Seam	Seam with deposited soil (infill), extremely weathered insitu rock (XW), or disoriented usually angular fragments of the host rock (crushed).
Shape	
Planar	Consistent orientation.
Curved	Gradual change in orientation.
Undulating	Wavy surface.
Stepped	One or more well defined steps.
Irregular	Many sharp changes in orientation.
Roughness	
Polished	Shiny smooth surface.
Slickensided	Grooved or striated surface, usually polished.
Smooth	Smooth to touch. Few or no surface irregularities.
Rough	Many small surface irregularities (amplitude generally <1mm). Feels like fine to coarse sandpaper.
Very Rough	Many large surface irregularities, amplitude generally >1mm. Feels like very coarse sandpaper.
Coating	
Clean	No visible coating or discolouring.
Stained	No visible coating but surfaces are discolored.
Veneer	A visible coating of soil or mineral, too thin to measure; may be patchy
Coating	Visible coating =1mm thick. Thicker soil material described as seam.

Excavation Log

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EX no:	**TP01**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	20.1.2021
principal:		**finished:**	20.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1493.07 m
dimensions:	**E:** 616527m **N:** 5959693m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1493.0			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
					0.375		SC	Clayey SAND, fine grained, brown.		Wp?	300	RESIDUAL.
					0.5							
				1492.5	0.55		SP	SAND, medium to coarse grained, light grey/ brown, trace of fine-grained sand, assessed as completely weathered granite bedrock.			330	GRANITE BEDROCK.
					1.0							
				1492.0								
					1.5							
				1491.5	1.5			Excavator reached target depth @ 1.5m on inferred Class 5 Granite bedrock. Excavation No: TP01 terminated at 1.5m				
					2.0							
				1491.0								
					2.5							
				1490.5								
					3.0							

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Excavation Log

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EX no:	TP02
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	20.1.2021
principal:		**finished:**	20.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1483.26 m
dimensions:	E: 616578m N: 5959708m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1483.0			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
					0.25		GRANITE	GRANITE, blocky mixed with silty sand matrix, pieces up to 150mm in size.				GRANITE BEDROCK.
					0.5							
				1482.5	0.6			Excavator reached practical refusal @ 0.6m on Granite bedrock. Excavation No: TP02 terminated at 0.6m				
					1.0							
				1482.0								
					1.5							
				1481.5								
					2.0							
				1481.0								
					2.5							
				1480.5								
					3.0							

Refer to Information Sheets for Terms and Symbols

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Excavation Log

EX no:	TP03
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	started:	20.1.2021
principal:		finished:	20.1.2021
project:	Proposed Alpine Coaster	logged:	MAB
location:	Thredbo NSW	checked:	AT
equipment:		RL surface:	1478.39 m
dimensions:	E: 616601m N: 5959710m	datum:	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed					SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		RESIDUAL.
				1478.0	0.35		SP	SAND, medium to coarse grained, light grey/ brown, trace of rocks.		VD		RESIDUAL.
					0.5							
				1477.5	0.75		GRANITE	GRANITE, blocky in silty sand matrix, assessed as completely to extremely weathered granite bedrock.				GRANITE BEDROCK.
					1.0			Excavator reached practical refusal @ 1.0m on Granite bedrock. Excavation No: TP03 terminated at 1m				
				1477.0	1.5							
				1476.5	2.0							
				1476.0	2.5							
				1475.5	3.0							

Refer to Information Sheets for Terms and Symbols

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Excavation Log

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EX no:	**TP04**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	20.1.2021
principal:		**finished:**	20.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1486.5 m
dimensions:	**E:** 616552m **N:** 5959663m	**datum:**	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed					SM	TOPSOIL, Silty, Clayey SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
				1486.0	0.4 / 0.5		SP	SAND with some fine to coarse gravel (granite), medium to coarse grained, light grey/ brown, trace of fine- grained sand.		VD		GRANITE BEDROCK.
				1485.5	0.8 / 1.0			Excavator reached target depth @ 0.8m on inferred extremely weathered granite bedrock. Excavation No: TP04 terminated at 0.8m				
				1485.0	1.5							
				1484.5	2.0							
				1484.0	2.5							
				1483.5	3.0							

Refer to Information Sheets for Terms and Symbols

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Excavation Log

EX no:	TP05
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	started:	20.1.2021
principal:		finished:	20.1.2021
project:	Proposed Alpine Coaster	logged:	MAB
location:	Thredbo NSW	checked:	AT
equipment:		RL surface:	1446.198 m
dimensions:	E: 616654m N: 5959690m	datum:	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N			1446.0	0.0–0.375		SM	TOPSOIL, Silty, Clayey SAND, fine grained, dark grey/ brown, trace of green roots.	W	L		TOPSOIL.
		Slow inflow (at 0.5)			0.375–0.55		SC	Gravelly, clayey SAND, fine grained, dark grey/ brown, fine to coarse rounded gravel.				SLOPEWASH?
				1445.5	0.55–1.0		GRANITE	GRANITE, blocky with clayey sand matrix, up to 150mm in size, orange brown/ brown, variably weathered (completely weathered to extremely weathered).				GRANITE BEDROCK. VERY SLOW DIGGING.
				1445.0, 1444.5, 1444.0, 1443.5	1 (1.5, 2.0, 2.5, 3.0)			Excavator reached target depth @ 1.0m on Granite bedrock. Excavation No: TP05 terminated at 1m				

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Excavation Log

EX no:	**TP06**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	20.1.2021
principal:		**finished:**	20.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1451 m
dimensions:	**E:** 616648m **N:** 5959696m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	**material** soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	**structure and additional observations**
EX	N	None Observed			0.0–0.4		SM	TOPSOIL, Silty, Clayey SAND, fine grained, dark grey/ brown, trace of green roots.	D	L		TOPSOIL.
				1450.5	0.4 (0.5)		CL/ML	Silty CLAY/ Clayey SILT, medium plasticity, brown.	<Wp	St		SLOPEWASH.
					0.8		SM	Silty SAND, fine to coarse grained, grey/ brown.	M	VD		RESIDUAL.
				1450.0	1 (1.0)		SP	SAND, completely weathered, fine to coarse grained, grey/ light grey.				COMPLETELY WEATHERED GRANITE.
					1.2			GRANITE, extremely weathered, grey/ light grey.				EXTREMELY WEATHERED GRANITE.
				1449.5	1.5			Excavation No: TP06 terminated at 1.5m				
				1449.0	2.0							
				1448.5	2.5							
				1448.0	3.0							

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Excavation Log

EX no:	**TP07**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	20.1.2021
principal:		**finished:**	20.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1443.36 m
dimensions:	E: 616671m N: 5959677m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1443.0			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
					0.4 / 0.5		ML	Sandy, Clayey SILT, medium plasticity, brown.	>Wp	H		SLOPEWASH.
					0.6		CL	Sandy CLAY, medium plasticity, brown.				RESIDUAL.
				1442.5	0.75 / 1.0		SC	Clayey SAND, fine to coarse grained, grey/ brown, assessed as completely weathered granite bedrock.	M	VD		GRANITE BEDROCK.
				1442.0 / 1441.5 / 1441.0 / 1440.5	1.1 / 1.5 / 2.0 / 2.5 / 3.0			Excavator reached target depth @ 1.1m on inferred completely weathered granite bedrock. Excavation No: TP07 terminated at 1.1m				

Refer to Information Sheets for Terms and Symbols

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Excavation Log

EX no:	**TP08**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	20.1.2021
principal:		**finished:**	20.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1436.49 m
dimensions:	**E:** 616698m **N:** 5959660m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed			0.0–0.4		SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, numerous tree roots with occasional granite boulbers up to 0.5m in size.	M	L		TOPSOIL.
				1436.0 / 1435.5	0.4–1.4 (0.5, 1.0)		CL	Sandy CLAY, medium plasticity, orange brown, fine to medium grained sand.	Wp	VSt	220, 200	RESIDUAL.
				1435.0	1.4–1.5		SC	Clayey SAND, fine to medium grained, orange brown.	M	D		GRANITE BEDROCK.
				1434.5 / 1434.0 / 1433.5	1.5 (2.0, 2.5, 3.0)			Excavator reached target depth @ 1.5m on inferred completely weathered granite bedrock. Excavation No: TP08 terminated at 1.5m				

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Excavation Log

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EX no:	TP09
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	started:	20.1.2021
principal:		finished:	20.1.2021
project:	Proposed Alpine Coaster	logged:	MAB
location:	Thredbo NSW	checked:	AT
equipment:		RL surface:	1434 m
dimensions:	E: 616736m N: 5959620m	datum:	AHD

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed			0.0–0.5		SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
				1433.5	0.5–1.5		CL	Sandy CLAY, medium plasticity, orange brown.	Wp	VSt		RESIDUAL.
				1433.0	1.0							
				1432.5	1.5			Excavator reached target depth @ 1.8m on very stiff clay. Excavation No: TP09 terminated at 1.5m				
				1432.0	2.0							
				1431.5	2.5							
				1431.0	3.0							

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Excavation Log

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EX no:	TP10
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	20.1.2021
principal:		**finished:**	20.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1413.5 m
dimensions:	**E:** 616748m **N:** 5959609m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed			0.0–0.3		SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
				1413.0	0.3–0.7 (0.5)			GRANITE with cobbles, boulders up to 400mm in size (flat) in clayey sand matrix.				GRANITE BEDROCK.
				1412.5 / 1412.0 / 1411.5 / 1411.0 / 1410.5	0.7 (1.0, 1.5, 2.0, 2.5, 3.0)			Excavator reached practical refusal @ 0.7m on inferred granite bedrock. Excavation No: TP10 terminated at 0.7m				

Refer to Information Sheets for Terms and Symbols

Excavation Log - Revision 9

Excavation Log

assetgeoenviro

EX no:	TP11
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	started:	20.1.2021
principal:		finished:	20.1.2021
project:	Proposed Alpine Coaster	logged:	MAB
location:	Thredbo NSW	checked:	AT
equipment:		RL surface:	1406.71 m
dimensions:	E: 616748m N: 5959589m	datum:	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1406.5			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
				1406.0	0.3 / 0.5 / 1.0		SC	Sandy CLAY, medium plasticiy, orange brown, fine to medium grained sand.	Wp	VSt	200, 250	RESIDUAL.
				1405.5 / 1405.0 / 1404.5 / 1404.0	1.1 / 1.5 / 2.0 / 2.5 / 3.0			Excavator reached practical refusal @ 1.1 on inferred granite boulder. Excavation No: TP11 terminated at 1.1m				GRANITE BOULDER.

Refer to Information Sheets for Terms and Symbols

Excavation Log - Revision 9

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Excavation Log

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EX no:	TP12
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	20.1.2021
principal:		**finished:**	20.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1399.5 m
dimensions:	E: 616763m N: 5959569m	**datum:**	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed					SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	D-M	L		TOPSOIL.
				1399.0	0.4 / 0.5		GRANITE	GRANITE (remould to sand), medium to coarse grained, grey/light grey, assessed as completely to extremely weathered Granite.	M	VD		GRANITE BEDROCK.
				1398.5	1.0							
					1.1			Excavator reached target depth @ 1.1m on weathered Granite bedrock. Excavation No: TP12 terminated at 1.1m				
				1398.0	1.5							
				1397.5	2.0							
				1397.0	2.5							
				1396.5	3.0							

Refer to Information Sheets for Terms and Symbols

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Excavation Log

EX no:	**TP13**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	20.1.2021
principal:		**finished:**	20.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1381.8 m
dimensions:	**E:** 616786m **N:** 5959504m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1381.5			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
				1381.0, 1380.5	0.35, 0.5, 1.0, 1.5		SC	Sandy CLAY, medium plasticity, orange/ brown, fine to medium grained.	Wp	VSt	350, 360	SLOPEWASH?
				1380.0, 1379.5, 1379.0	1.5, 2.0, 2.5, 3.0			Excavator reached target depth @ 1.5m on very stiff clay. Excavation No: TP13 terminated at 1.5m				

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Excavation Log

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EX no:	TP14
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	started:	20.1.2021
principal:		finished:	20.1.2021
project:	Proposed Alpine Coaster	logged:	MAB
location:	Thredbo NSW	checked:	AT
equipment:		RL surface:	1377 m
dimensions:	E: 616809m N: 5959494m	datum:	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed			0.0–0.4		SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
				1376.5, 1376.0	0.4–1.4 (0.5, 1.0)		SC	Sandy CLAY, medium plasticity, orange/brown, fine to medium grained sand.	Wp	VSt		SLOPEWASH?
				1375.5, 1375.0, 1374.5, 1374.0	1.4 (1.5, 2.0, 2.5, 3.0)			Excavator reached target depth @ 1.4m on very stiff clay. Excavation No: TP14 terminated at 1.4m				

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Excavation Log

EX no:	TP15
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	started:	20.1.2021
principal:		finished:	20.1.2021
project:	Proposed Alpine Coaster	logged:	MAB
location:	Thredbo NSW	checked:	AT
equipment:		RL surface:	1369 m
dimensions:	E: 616837m N: 5959475m	datum:	AHD

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components)	moisture condition	consistency/ density index	hand penetrometer kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed			0.0–0.4		FILL	FILL, mixture of gravels and cobbles up to 150mm in size in a clayey sand/ sandy clay matrix, grey/ brown.	M	VD/H		FILL, WELL COMPACTED.
				1368.5	0.4–1.2		FILL	FILL, gravels with minimal cobbles up to 150mm in size in a sandy clay matrix, grey/ brown.		VSt		FILL, MODERATE TO WELL COMPACTED.
				1368.0 / 1367.5	1.2–1.7		ML	Sandy, Clayey SILT, dark grey, fine to medium grained sand, trace of organic matter.	>Wp	St-VSt	180; 230	ALLUVIUM.
				1367.0 / 1366.5 / 1366.0	1.7			Excavator reached target depth @ 1.7m on very stiff clay. Excavation No: TP15 terminated at 1.7m				

Depth scale: 0.5, 1.0, 1.5, 2.0, 2.5, 3.0

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Excavation Log

EX no:	TP16
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	started:	20.1.2021
principal:		finished:	20.1.2021
project:	Proposed Alpine Coaster	logged:	MAB
location:	Thredbo NSW	checked:	AT
equipment:		RL surface:	1367.06 m
dimensions:	E: 616849m N: 5959462m	datum:	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetrometer kPa (100 200 300 400)	structure and additional observations
EX/DCP	N	None Observed		1367.0	0.0		SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	VD / MD		TOPSOIL/FILL.
					0.3		SM/ML	Silty SAND/ Sandy SILT, fine grained, medium plasticity, boulder up to 350mm in size.		D / MD / D		FILL.
				1366.5	0.6		SC	Sandy CLAY, medium plasticity, mottled grey/ orange brown, fine to coarse gravel.	>Wp	St / VSt	× 295	
					0.9		--	ORGANIC LAYER.	M	MD		TOPSOIL.
				1366.0	1.1		SC	Clayey SAND, medium to coarse grained, light grey.		D / MD / D		RESIDUAL.
DCP				1365.5	1.6			Excavator reached target depth @ 1.6m on very dense sand.		VD / D / MD / D		
				1365.0 / 1364.5	2.0 / 2.5							
					3.0			DCP reached maximum depth @ 2.6m on dense sand due to limited steel rods. Excavation No: TP16 terminated at 2.6m				

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Excavation Log

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EX no:	**TP17**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT		**started:**	20.1.2021
principal:			**finished:**	20.1.2021
project:	Proposed Alpine Coaster		**logged:**	MAB
location:	Thredbo NSW		**checked:**	AT
equipment:			**RL surface:**	1365.01 m
dimensions:		**E:** 616857m **N:** 5959463m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX/DCP	N	None Observed			0.0		SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	VL		TOPSOIL.
										MD		
					0.3		SC/CL	Clayey SAND/ Sandy CLAY. fine to coarse grained sand, medium plasticity clay, grey/ brown.	M-W/ >>Wp			RESIDUAL.
										D		
				1364.5	0.5						130	
											225	
				1364.0	1.0					VD		
										D		
DCP					1.4			Excavator reached target depth @ 1.4m on very stiff clay.				
				1363.5	1.5					MD		
										D		
										MD		
										D		
				1363.0	2.0							
				1362.5	2.5					VD		
								DCP reached maximum depth @ 2.6m on very dense sand due to limited steel rods. Excavation No: TP17 terminated at 2.6m				
					3.0							

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Excavation Log

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EX no:	**TP18**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	21.1.2021
principal:		**finished:**	21.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1364.53 m
dimensions:	E: 616816m N: 5959432m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components)	moisture condition	consistency/ density index	hand penetrometer kPa	structure and additional observations
EX/DCP	N			1364.5	0.0–0.15		SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	MD		TOPSOIL/FILL.
					0.15–0.3		CL/ML	Silty CLAY/ Clayey SILT, medium plasticity, dark grey.	>Wp	F		TOPSOIL.
		Moderate Inflow (~1.0 m)		1364.0, 1363.5	0.3–1.5		CL/SC	Sandy CLAY & Clayey SAND, medium plasticity, fine to medium grained sand, orange/brown/ light grey.	>Wp; W (from ~1.0 m)	St; VSt; H; VSt; St	250 (0.5 m); 310 (0.65 m); 50 (1.1 m)	RESIDUAL.
DCP				1363.0, 1362.5	1.5–2.5			Excavator reached target depth @ 1.5m on very stiff clayey sand material.		St; VSt; St; VSt; H		
				1362.0	2.5			DCP reached maximum depth @ 2.5m on hard clay due to limited steel rods. Excavation No: TP18 terminated at 2.5m				

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Excavation Log

EX no:	**TP19**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	21.1.2021
principal:		**finished:**	21.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1367 m
dimensions:	**E:** 616801m **N:** 5959436m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed					--	Woodchip over geofabric.	--	--		Landscaping.
					0.075		--	FILL, mixture of sandy clay and fine to medium gravel.	>Wp	VSt		FILL (Well Compacted).
				1366.5	0.5		ML	Sandy, Clayey SILT, low plasticity, dark grey to black, numerous tree roots, trace of organic matter.			350	ALLUVIUM.
				1366.0	0.9 / 1.0		SP	SAND, medium to coarse grained, grey/ light grey, trace of fine-grained sand, excavates as coarse gravel with fine cobbles, assessed as completely weathered granite.	M	VD		GRANITE BEDROCK.
				1365.5	1.4 / 1.5		CL	Silty CLAY, medium plasticity, grey, assessed as completely weathered granite.	Wp	St	180 125	
				1365.0	2.0			Excavator reached target depth @ 2.0m on stiff clay. Excavation No: TP19 terminated at 2m				
				1364.5	2.5							
				1364.0	3.0							

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Excavation Log

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EX no:	TP20
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	started:	21.1.2021
principal:		finished:	21.1.2021
project:	Proposed Alpine Coaster	logged:	MAB
location:	Thredbo NSW	checked:	AT
equipment:		RL surface:	1368.4 m
dimensions:	E: 616778m N: 5959455m	datum:	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetrometer kPa (100 200 300 400)	structure and additional observations
EX/DCP	N			1368.0 (0.5), 1367.5 (1.0)	0.0–1.3		SC/CL	FILL, mixture of clayey sand & sandy clay with some fine to coarse gravels.	M	H; VSt; H		FILL - WELL COMPACTED. Rounded grey gravel (trench?) at western side of pit, moved East. Stormwater pipe, moved East.
		Rapid Inflow		1367.0 (1.5), 1366.5 (2.0)	1.3–2.0		SC	Clayey SAND, medium to coarse grained.	W	VD; D; MD; D; VD		Residual/ Completely weathered Granite.
DCP				1366.0 (2.5)	2.0–2.6			Excavator reached maximum depth @ 2.0m due to side walls collapsing belwo 1.3m.		VD; D; MD; L		
				1365.5 (3.0)	2.6			DCP reached maximum depth @ 2.6m on loose sand due to limited steel rods. Excavation No: TP20 terminated at 2.6m				

Excavation Log

EX no:	**TP21**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	21.1.2021
principal:		**finished:**	21.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1372.56 m
dimensions:	**E:** 616706m **N:** 5959475m	**datum:**	AHD

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1372.5			SM	TOPSOIL/FILL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL/FILL
					0.3		SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.				TOPSOIL
				1372.0	0.6		CL	Silty CLAY, brown, some fine to medium gravel.	>Wp	St/VSt	220 180	RESIDUAL.
				1371.5	1.2		CL	Silty CLAY, light brown, fine to medium grained sand.			220	
				1371.0	1.5			Excavator reached target depth @ 1.5m on very stiff clay. Excavation No: TP21 terminated at 1.5m				
				1370.5								
				1370.0	3.0							

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Excavation Log

EX no:	**TP22**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	21.1.2021
principal:		**finished:**	21.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1457.18 m
dimensions:	E: 616611m N: 5959613m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1457.0			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	D-M	L		TOPSOIL.
				1456.5	0.5		SP	SAND, medium to coarse grained, some fine-grained sand, granite boulder up to 0.3m in size, assessed as completely weathered granite.	M	VD		GRANITE BEDROCK.
				1456.0 1455.5 1455.0 1454.5	0.9 (1.0, 1.5, 2.0, 2.5, 3.0)			Excavator reached practical refusal @ 0.9m on inferred granite bedrock. Excavation No: TP22 terminated at 0.9m				

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Excavation Log

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EX no:	TP23
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	21.1.2021
principal:		**finished:**	21.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1444.77 m
dimensions:	E: N:	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1444.5			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	M	L		TOPSOIL.
				1444.0	0.3 (0.5, 1.0)		CL	Silty CLAY, medium plasticity, dark brown.	<Wp	VSt		SLOPEWASH.
				1443.5, 1443.0	1.1 (1.5)		SP	SAND, medium to coarse grained, some fine grained sand, assessed as completely weathered granite.	M	D		GRANITE BEDROCK.
				1442.5, 1442.0	1.8 (2.0, 2.5, 3.0)			Excavator reached target depth @ 1.8m on dense sand. Excavation No: TP23 terminated at 1.8m				

Refer to Information Sheets for Terms and Symbols

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Excavation Log

EX no:	TP24
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	21.1.2021
principal:		**finished:**	21.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1429.25 m
dimensions:	E: 616647m N: 5959579m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetrometer kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1429.0	0.0–0.4		SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	D-M	L		TOPSOIL.
				1428.5	0.4–1.0 (0.5)		CL	Silty CLAY, medium plasticity, dark brown.	<Wp	VSt		SLOPEWASH.
					1.0–1.2		SP	SAND, medium to coarse grained, some fine grained sand.	M	VD		GRANITE BEDROCK.
				1428.0	1.2			Excavator reached target depth @ 1.2m on inferred completely weathered granite. Excavation No: TP24 terminated at 1.2m				
				1427.5, 1427.0, 1426.5	1.5, 2.0, 2.5, 3.0							

Refer to Information Sheets for Terms and Symbols

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Excavation Log

EX no:	**TP25**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	21.1.2021
principal:		**finished:**	21.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1419.7 m
dimensions:	E: N:	**datum:**	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetrometer kPa (100 200 300 400)	structure and additional observations
EX	N	None Observed		1419.5			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, trace of green roots.	D-M	L		TOPSOIL.
				1419.0	0.4		CL	Silty CLAY, medium plasticity, dark brown.	<Wp	VSt		SLOPEWASH.
				1418.5	0.75		SC	Clayey SAND with flat Granite cobbles up to 200m in size.	M	VD		RESIDUAL.
				1418.0, 1417.5, 1417.0	1.2			Excavator reached target depth @ 1.2m on very dense sand. Excavation No: TP25 terminated at 1.2m				

Depth scale: 0.5, 1.0, 1.5, 2.0, 2.5, 3.0

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Excavation Log

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EX no:	HA01
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	started:	22.1.2021
principal:		finished:	22.1.2021
project:	Proposed Alpine Coaster	logged:	MAB
location:	Thredbo NSW	checked:	AT
equipment:		RL surface:	1471.7 m
dimensions:	E: 616593m N: 5959639m	datum:	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetrometer kPa (100 200 300 400)	structure and additional observations
HA/DCP	N	None Observed		1471.5			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, roots.	M	VL / MD / L		TOPSOIL.
					0.4 / 0.5		ML	Clayey SILT, medium plasticity, dark brown, some fine grained sand.	<Wp	St / F		SLOPEWASH.
				1471.0	0.6		SM	Clayey, Silty, SAND, medium to coarse grained, light grey/ brown.	M	MD		GRANITE BEDROCK.
DCP				1470.5 / 1470.0	0.7 / 1.0 / 1.5			Hand Auger reached practical refusal @ 0.7m on medium dense sand.		L / MD / D / VD / D		
					2.0			DCP bouncing @ 1.7m on inferred granite bedrock. Excavation No: HA01 terminated at 1.7m				

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Excavation Log

EX no:	**HA02**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	22.1.2021
principal:		**finished:**	22.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1403.68 m
dimensions:	**E:** 616694m **N:** 5959530m	**datum:**	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	**material** soil type: plasticity or particle characteristics, colour, secondary and minor components.	moisture condition	consistency/ density index	hand penetro-meter kPa 100 200 300 400	**structure and additional observations**
HA/DCP	N	None Observed		1403.5			SM	TOPSOIL, Silty SAND, fine grained, dark grey/ brown, roots.	M	VL / L		TOPSOIL.
					0.4 / 0.5		ML	Clayey SILT, medium plasticity, dark brown, some fine grained sand.	<Wp	F / VSt		SLOPEWASH.
				1403.0 / 1402.5	0.6 / 1.0		SM	Clayey Silty SAND, medium to coarse grained, light grey/ brown.	M	D / L / D / MD / D		GRANITE BEDROCK.
DCP				1402.0	1.2 / 1.5			Hand Auger reached practical refusal @ 1.2m on dense sand.				
					2.0			DCP bouncing @ 1.7m on inferred Granite bedrock. Excavation No: HA02 terminated at 1.7m				

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Excavation Log

EX no:	**HA03**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	22.1.2021
principal:		**finished:**	22.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1389.5 m
dimensions:	E: 616273m N: 5959501m	**datum:**	AHD

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
HA/DCP	N	None Observed			0.0–0.4		SM	TOPSOIL, Silty SAND, fine to medium grained, dark grey/ brown, grass roots.	M	L / D / VD		TOPSOIL.
				1389.0	0.4–0.6 (0.5)		ML	Clayey SILT, medium plasticity, dark brown, some fine grained sand.	<Wp	St / VSt		SLOPEWASH.
DCP				1388.5	0.6–1.0			Hand Auger refusal @ 0.6m on sandy gravels.	M	D / VD		SANDY GRAVELS.
				1388.0 / 1387.5	1.0–2.0 (1.5, 2.0)			DCP reached practical refusal @ 1.0m on very dense sand. Excavation No: HA03 terminated at 1m				

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Excavation Log

EX no:	HA04
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	22.1.2021
principal:		**finished:**	22.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1459 m
dimensions:	E: 616620m N: 5959686m	**datum:**	AHD

excavation information | **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components)	moisture condition	consistency/ density index	hand penetrometer kPa (100 200 300 400)	structure and additional observations
HA/DCP	N	None Observed			0.0		TOPSOIL	Gravelly, Silty SAND, fine grained, fine to medium gravel, grass roots to 0.2m,	M	VL		TOPSOIL.
										D		
										L		
				1458.5	0.5					D		
										L		
					0.65		SP	SAND, medium to coarse grained, light brown, trace of fine grained sand, some fine to medium gravels.				RESIDUAL.
										D		
										MD		
										VD		
				1458.0	1.0							
DCP					1.15			Hand Auger reached practical refusal @ 1.15m on inferred completely weathered granite.				GRANITE BEDROCK.
										D		
										VD		
				1457.5	1.5							
								DCP reached practical refusal @ 1.55m on inferred completely weathered granite bedrock. Excavation No: HA04 terminated at 1.55m				
				1457.0	2.0							

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Excavation Log

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EX no:	**HA05**
sheet:	1 of 1
job no.:	5920

client:	KT-EVT	**started:**	22.1.2021
principal:		**finished:**	22.1.2021
project:	Proposed Alpine Coaster	**logged:**	MAB
location:	Thredbo NSW	**checked:**	AT
equipment:		**RL surface:**	1394.81 m
dimensions:	**E:** 616744m **N:** 5959551m	**datum:**	AHD

excavation information / **material information**

method	support	water	notes samples, tests, etc	RL	depth metres	graphic log	USCS symbol	material (soil type: plasticity or particle characteristics, colour, secondary and minor components.)	moisture condition	consistency/ density index	hand penetro-meter kPa (100 200 300 400)	structure and additional observations
HA/DCP	N	None Observed		1394.5	0.0–0.5		ML	TOPSOIL, Silt, medium plasticity, dark grey.	>>Wp	VS / VSt / St / H		TOPSOIL.
DCP					0.5			Hand Auger reached practical refusal @ 0.5m on cobbles/gravel layer.				COBBLES/GRAVELS.
				1394.0				DCP reached practical refusal @ 0.7m on cobbles/gravel layer. Excavation No: HA05 terminated at 0.7m				
				1393.5	1.0							
				1393.0	1.5							
					2.0							

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Dynamic Cone Penetrometer

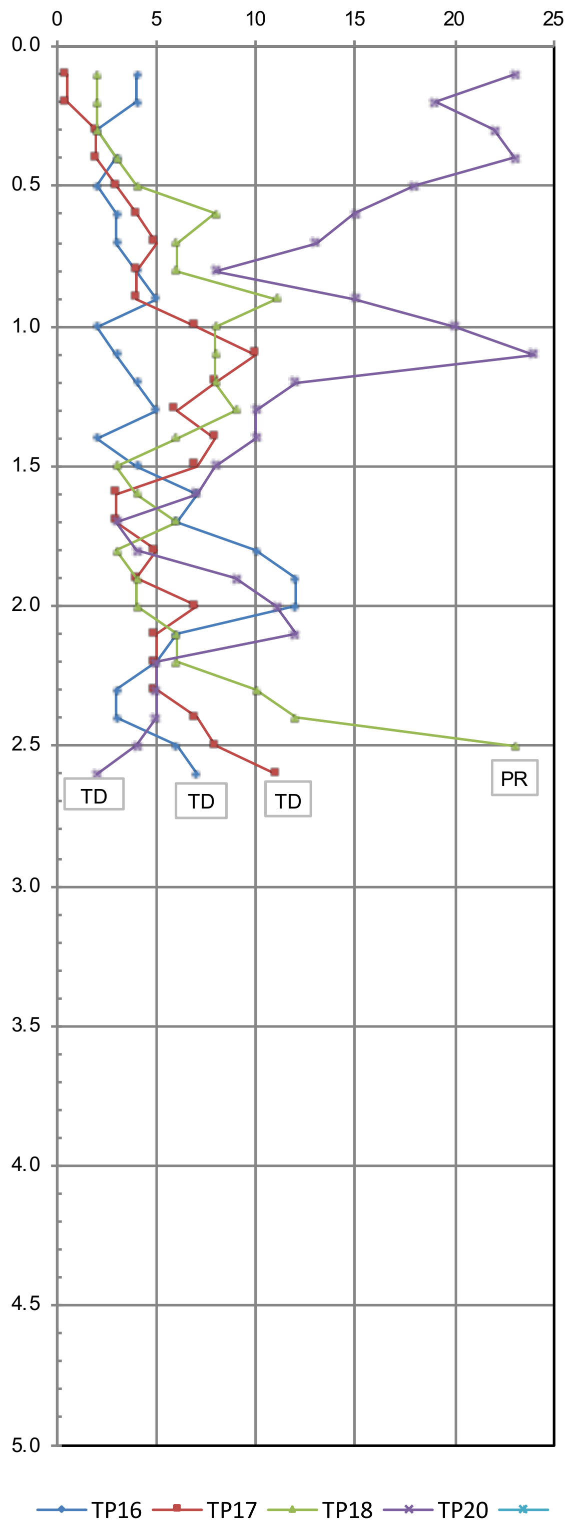
Job No: 5920

client:	KT-EVT	**started:**	21.01.2021
principal:		**finished:**	22.01.2021
project:	Proposed Alpine Coaster	**logged:**	AT
location:	Thredbo NSW	**checked:**	MAB
equipment:	9kg hammer, 510mm drop, cone tip		
standard:	AS1289.6.3.2-1997		

Test Results (blows / 100mm)

Depth (m)	TP16	TP17	TP18	TP20	
0.00 – 0.10	4	0.5	2	23	
0.10 – 0.20	4	0.5	2	19	
0.20 – 0.30	2	2	2	22	
0.30 – 0.40	3	2	3	23	
0.40 – 0.50	2	3	4	18	
0.50 – 0.60	3	4	8	15	
0.60 – 0.70	3	5	6	13	
0.70 – 0.80	4	4	6	8	
0.80 – 0.90	5	4	11	15	
0.90 – 1.00	2	7	8	20	
1.00 – 1.10	3	10	8	24	
1.10 – 1.20	4	8	8	12	
1.20 – 1.30	5	6	9	10	
1.30 – 1.40	2	8	6	10	
1.40 – 1.50	4	7	3	8	
1.50 – 1.60	7	3	4	7	
1.60 – 1.70	6	3	6	3	
1.70 – 1.80	10	5	3	4	
1.80 – 1.90	12	4	4	9	
1.90 – 2.00	12	7	4	11	
2.00 – 2.10	6	5	6	12	
2.10 – 2.20	5	5	6	5	
2.20 – 2.30	3	5	10	5	
2.30 – 2.40	3	7	12	5	
2.40 – 2.50	6	8	23	4	
2.50 – 2.60	7	11	PR	2	
2.60 – 2.70	TD	TD		TD	
2.70 – 2.80					
2.80 – 2.90					
2.90 – 3.00					
3.00 – 3.10					
3.10 – 3.20					
3.20 – 3.30					
3.30 – 3.40					
3.40 – 3.50					
3.50 – 3.60					
3.60 – 3.70					
3.70 – 3.80					
3.80 – 3.90					
3.90 – 4.00					
4.00 – 4.10					
4.10 – 4.20					
4.20 – 4.30					
4.30 – 4.40					
4.40 – 4.50					
4.50 – 4.60					
4.60 – 4.70					
4.70 – 4.80					
4.80 – 4.90					
4.90 – 5.00					

Plot (blows / 100mm vs depth)



Notes:
RL = ground surface level (m) AHD
TD = target depth, PR = practical refusal (15+ blows per 100mm), SR = "solid" refusal (no further penetration and "solid" ringing sound from slide hammer)

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Dynamic Cone Penetrometer

Sheet: 1 of 1

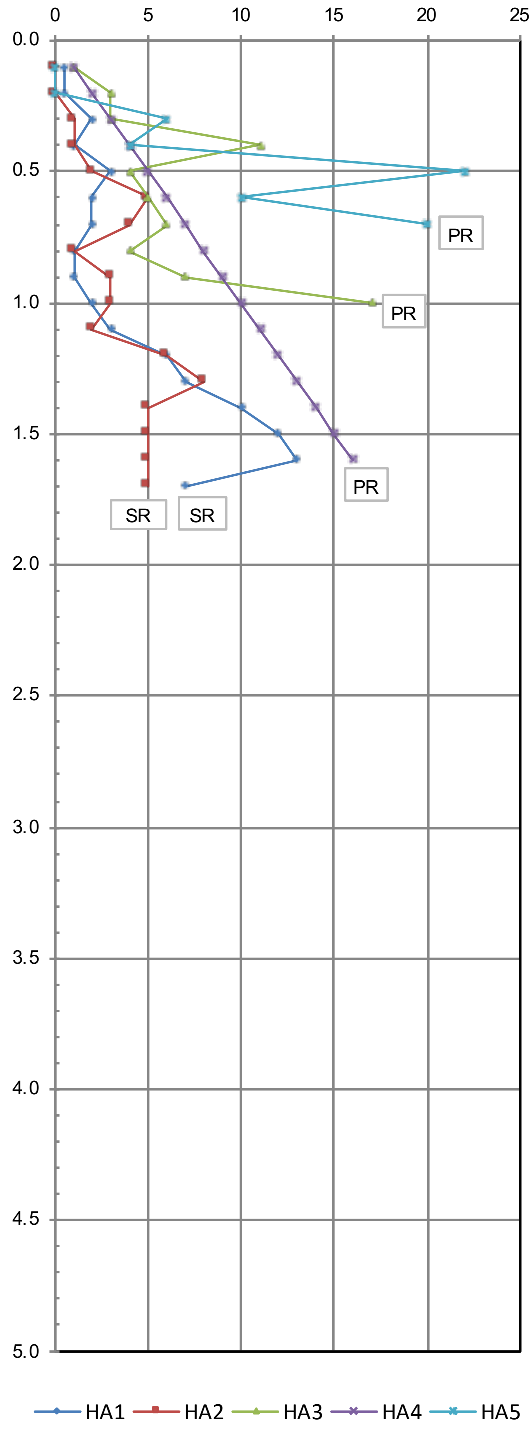
Job No: 5920

client:	KT-EVT	**started:**	21.01.2021
principal:		**finished:**	22.01.2021
project:	Proposed Alpine Coaster	**logged:**	AT
location:	Thredbo NSW	**checked:**	MAB
equipment:	9kg hammer, 510mm drop, cone tip		
standard:	AS1289.6.3.2-1997		

Test Results (blows / 100mm)

Depth (m)	HA1	HA2	HA3	HA4	HA5
	Peg 42	New Peg 24		Peg 76	Peg 135
0.00 – 0.10	0.5	0	1	0.5	0
0.10 – 0.20	0.5	0	3	0.5	0
0.20 – 0.30	2	1	3	3	6
0.30 – 0.40	1	1	11	1	4
0.40 – 0.50	3	2	4	1	22
0.50 – 0.60	2	5	5	3	10
0.60 – 0.70	2	4	6	1	20
0.70 – 0.80	1	1	4	3	PR
0.80 – 0.90	1	3	7	2	
0.90 – 1.00	2	3	17	10	
1.00 – 1.10	3	2	PR	10	
1.10 – 1.20	6	6		16	
1.20 – 1.30	7	8		8	
1.30 – 1.40	10	5		10	
1.40 – 1.50	12	5		12	
1.50 – 1.60	13	5		15/50mm	
1.60 – 1.70	7	5		PR	
1.70 – 1.80	SR	SR			
1.80 – 1.90					
1.90 – 2.00					
2.00 – 2.10					
2.10 – 2.20					
2.20 – 2.30					
2.30 – 2.40					
2.40 – 2.50					
2.50 – 2.60					
2.60 – 2.70					
2.70 – 2.80					
2.80 – 2.90					
2.90 – 3.00					
3.00 – 3.10					
3.10 – 3.20					
3.20 – 3.30					
3.30 – 3.40					
3.40 – 3.50					
3.50 – 3.60					
3.60 – 3.70					
3.70 – 3.80					
3.80 – 3.90					
3.90 – 4.00					
4.00 – 4.10					
4.10 – 4.20					
4.20 – 4.30					
4.30 – 4.40					
4.40 – 4.50					
4.50 – 4.60					
4.60 – 4.70					
4.70 – 4.80					
4.80 – 4.90					
4.90 – 5.00					

Plot (blows / 100mm vs depth)



Notes:
RL = ground surface level (m) AHD
TD = target depth, PR = practical refusal (15+ blows per 100mm), SR = "solid" refusal
(no further penetration and "solid" ringing sound from slide hammer)

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Appendix C

Site Photos

Photo 1

North east of exit area, looking southwest towards circle construction

Photo 2

From Photo 1 location looking west

Photo 3

From Photo 1 location looking northwest

Photo 4

View from tunnel location looking upslope to northwest

Photo 5

View from Photo 4 location looking north

Photo 6

View from Photo 4 location looking northeast

Photo 7

View from Photo 4 location looking east

Photo 8

View near TP13 location looking north

Photo 9

View near TP13 location looking east

Photo 10

View near TP13 location looking southeast

Photo 11

View near TP13 location looking south

Photo 12

View near TP13 location looking southwest

Photo 13

View near TP22 looking downslope (south east)

Photo 14

View near TP22 looking upslope (north west)

Photo 15

View near TP07 looking upslope (north west)

Photo 16

View near TP06 looking downslope towards TP05 (south east)

Photo 17

View upslope of TP06 looking upslope (north west)

Photo 18

View at top of coaster incline south west of TP01 looking downslope (south east)

Photo 19

View north east of TP01 looking south west towards top of incline.

Photo 20

View near TP01 looking upslope (north west)